



**FORM 1**  
**CERTIFICATE OF SEISMIC PERFORMANCE LEVEL**

- UC-Designed & Constructed Facility**  
 **Campus-Acquired or Leased Facility**

**BUILDING DATA**

Building Name: Mission Hall Building  
Address: 550 16th St., San Francisco  
Site location coordinates: Latitude 37.7676 Longitudinal -122.3903

**UCOP SEISMIC PERFORMANCE LEVEL (OR "RATING"): III**

ASCE 41-17 Model Building Type:

- a. Longitudinal Direction: C2/S2: Concrete Shear Walls with Ordinary Steel Concentrically Braced Frames at Penthouse
- b. Transverse Direction: C2/S2: Concrete Shear Walls with Ordinary Steel Concentrically Braced Frames at Penthouse

Gross Square Footage: 276,385  
Number of stories *above* grade: 7  
Number of basement stories *below* grade: 0

Year Original Building was Constructed: 2014  
Original Building Design Code & Year: CBC-2010  
Retrofit Building Design Code & Code (if applicable): NA

**SITE INFORMATION**

Site Class: D Basis: (Rutherford & Chekene, 2/9/2015, S-002)  
Geologic Hazards:  
Fault Rupture: No Basis: UCSF Presumptive Buildings – Geotechnical Assessment, Egan (2019)  
Liquefaction: No Basis: UCSF Presumptive Buildings – Geotechnical Assessment, Egan (2019)  
Landslide: No Basis: UCSF Presumptive Buildings – Geotechnical Assessment, Egan (2019)

**ATTACHMENT**

Original Structural Drawings: (UCSF Block 25A, Rutherford & Chekene, 2/9/2015, S-002) or  
Seismic Evaluation: NA  
Retrofit Structural Drawings: NA



## CERTIFICATION & PRESUMPTIVE RATING VERIFICATION STATEMENT

I, [Maryann T. Phipps](#), a California-licensed structural engineer, am responsible for the completion of this certificate, and I have no ownership interest in the property identified above. My scope of review to support the completion of this certificate included both of the following ("No" responses must include an explanation):

- a) the review of structural drawings indicating that they are as-built or record drawings, or that they otherwise are the basis for the construction of the building:  Yes  No
- b) visiting the building to verify the observable existing conditions are reasonably consistent with those shown on the structural drawings:  Yes  No

Based on my review, I have verified that the UCOP Seismic Performance Level (SPL) is presumptively permitted by the following UC Seismic Program Guidebook provision (choose one of the following):

- 1) Contract documents indicate that the original design and construction of the aforementioned building is in accordance with the benchmark design code year (or later) building code seismic design provisions for UBC or IBC listed in Table 1 below.
- 2) The existing SPL rating is based on an acceptable basis of seismic evaluation completed in 2006 or later.
- 3) Contract documents indicate that a comprehensive<sup>1</sup> building seismic retrofit design was fully-constructed with an engineered design based on the 1997 UBC/1998 **or later** CBC, and (choose one of the following):
  - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1E (or BSE-R) and BSE-2E (or BSE-C) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 CBC **or later** for EXISTING buildings, and is presumptively assigned an SPL rating of IV.
  - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1 (or BSE-1N) and BSE-2 (or BSE-2N) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 **or later** CBC for NEW buildings, and is presumptively assigned an SPL rating of III.
  - the retrofit project was not completed by the UC campus following UC policies, and is presumptively assigned an SPL rating of IV.

<sup>1</sup> A comprehensive retrofit addresses the entire building structural system as indicated by the associated seismic evaluation, as opposed to addressing selective portions of the structural system.

Campus: UCSF  
Building Name: Mission Hall Building  
CAAN ID: 2281  
Auxiliary Building ID: NA



UNIVERSITY  
OF  
CALIFORNIA

Date: 8/16/2019

**CERTIFICATION SIGNATURE**

Maryann T. Phipps  
Print Name

President  
Title

S2995  
CA Professional Registration No.

6/30/2020  
License Expiration Date

*Maryann T. Phipps*  
Signature

8/16/2019  
Date

AFFIX SEAL HERE



Estructure, (510) 235-3116, 1144 65th St Suite A, Oakland  
Firm Name, Phone Number, and Address



**Table 1: Benchmark Building Codes and Standards**

Building Type <sup>a,b</sup>	Building Seismic Design Provisions	
	UBC	IBC
Wood frame, wood shear panels (Types W1 and W2)	1976	2000
Wood frame, wood shear panels (Type W1a)	1976	2000
Steel moment-resisting frame (Types S1 and S1a)	1997	2000
Steel concentrically braced frame (Types S2 and S2a)	1997	2000
Steel eccentrically braced frame (Types S2 and S2a)	1988 <sup>g</sup>	2000
Buckling-restrained braced frame (Types S2 and S2a)	<i>f</i>	2006
Metal building frames (Type S3)	<i>f</i>	2000
Steel frame with concrete shear walls (Type S4)	1994	2000
Steel frame with URM infill (Types S5 and S5a)	<i>f</i>	2000
Steel plate shear wall (Type S6)	<i>f</i>	2006
Cold-formed steel light-frame construction—shear wall system (Type CFS1)	1997 <sup>h</sup>	2000
Cold-formed steel light-frame construction—strap-braced wall system (Type CFS2)	<i>f</i>	2003
Reinforced concrete moment-resisting frame (Type C1) <sup>i</sup>	1994	2000
Reinforced concrete shear walls (Types C2 and C2a)	1994	2000
Concrete frame with URM infill (Types C3 and C3a)	<i>f</i>	<i>f</i>
Tilt-up concrete (Types PC1 and PC1a)	1997	2000
Precast concrete frame (Types PC2 and PC2a)	<i>f</i>	2000
Reinforced masonry (Type RM1)	1997	2000
Reinforced masonry (Type RM2)	1994	2000
Unreinforced masonry (Type URM)	<i>f</i>	<i>f</i>
Unreinforced masonry (Type URMa)	<i>f</i>	<i>f</i>
Seismic isolation or passive dissipation	1991	2000

Note: This table has been adapted from ASCE 41-17 Table 3-2. Benchmark Building Codes and Standards for Life Safety Structural Performed at BSE-1E.

Note: UBC = Uniform Building Code. IBC = International Building Code.

<sup>a</sup> Building type refers to one of the common building types defined in Table 3-1 of ASCE 41-17.

<sup>b</sup> Buildings on hillside sites shall not be considered Benchmark Buildings.

<sup>c</sup> not used

<sup>d</sup> not used

<sup>e</sup> not used

<sup>f</sup> No benchmark year; buildings shall be evaluated in accordance with Section III.J.

<sup>g</sup> Steel eccentrically braced frames with links adjacent to columns shall comply with the 1994 UBC Emergency Provisions, published September/October 1994, or subsequent requirements.

<sup>h</sup> Cold-formed steel shear walls with wood structural panels only.

<sup>i</sup> Flat slab concrete moment frames shall not be considered Benchmark Buildings.

GENERAL

- A. FOR MORE DETAILED INFORMATION, SEE PROJECT SPECIFICATIONS...
B. ALL CONSTRUCTION SHALL CONFORM TO THE CALIFORNIA BUILDING CODE (CBC), TITLE 24, 2010 EDITION...
C. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE JOB SITE BEFORE COMMENCING WORK...

DIMENSIONAL CONTROL

- A. DO NOT USE SCALED DIMENSIONS, WHERE NO WRITTEN DIMENSION IS PROVIDED, CONSULT WITH THE DESIGN-BUILDER FOR CLARIFICATION BEFORE PROCEEDING WITH THE WORK...
B. THE FOLLOWING MEMBERS CAN BE LOCATED WITHOUT WRITTEN DIMENSIONS:

- 1. CENTER MEMBERS ON GRID LINES THAT ARE SHOWN LOCATED ON GRID LINES.
2. SPACE MEMBERS EQUALLY BETWEEN MEMBERS ON GRID LINES OR MEMBERS OTHERWISE LOCATED, WHERE MEMBERS ARE SHOWN EQUALLY SPACED.

ELEVATIONS NOTED ON THE STRUCTURAL DRAWINGS USE THE FOLLOWING CONVENTIONS:

- 1. ACTUAL ELEVATIONS ARE DESIGNATED IN FEET AND INCHES (EXAMPLE: EL. 105'-0"), REFERENCING NGVD DATUM.
2. FEATURES THAT ARE CLOSELY RELATED TO OTHER SIMILAR ELEMENTS AT INDIVIDUAL FLOORS, ARE REFERENCED TO THE ELEVATION OF THE TYPICAL ELEMENT AT THAT LEVEL...

DIMENSION POINTS ARE AS FOLLOWS, UNLESS OTHERWISE INDICATED:

- 1. TYPICAL, U.O.N.: CENTERLINE.
2. WALLS: FACE OF CONCRETE OR FACE OF STUDS.
3. STEEL ANGLES AND CHANNELS: FACE.
4. FLAT FRAMING: TOP OF STEEL OR TOP OF CONCRETE.

COORDINATION OF DOCUMENTS

- A. ALTHOUGH WATERPROOFING AND DRAINAGE ITEMS ARE SOMETIMES PICTURED ON THE STRUCTURAL DRAWINGS FOR REFERENCE, THESE ITEMS ARE THE DESIGN RESPONSIBILITY OF OTHERS AND ARE DETAILED AND/OR SPECIFIED ELSEWHERE.
B. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND DETAILS OF NON-BEARING INTERIOR AND EXTERIOR WALL CONSTRUCTION.
C. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR LOCATIONS AND SIZES OF THE FOLLOWING ITEMS:

- 1. ANCHOR BOLTS, INSERTS AND HANGERS FOR ATTACHMENT AND BRACING OF NONSTRUCTURAL BUILDING COMPONENTS TO THE BUILDING STRUCTURE.
2. ITEMS EMBEDDED IN STRUCTURAL ELEMENTS, INCLUDING DRAINS, SLEEVES, CONDUITS, AND BOXES.
3. OPENINGS IN SLAB FOR INDIVIDUAL PIPES SMALLER THAN 6" IN DIAMETER.

- D. ITEMS THAT ARE NOT SHOWN ON STRUCTURAL DRAWINGS, BUT AFFECT STRUCTURAL ELEMENTS, SHALL BE SUBJECT TO LIMITATIONS OF THE STRUCTURAL DETAILS LISTED BELOW, EXCEPT AS APPROVED BY DESIGN-BUILDER

Table with 2 columns: ITEM, DETAIL REFERENCE. Rows include PENETRATIONS THROUGH FOOTINGS AND GRADE BEAMS (9/S-300) and PENETRATIONS THROUGH WALLS AND SLABS (3/S-301).

- E. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND DETAILS OF MISCELLANEOUS STEEL ITEMS, INCLUDING BUT NOT LIMITED TO:

- 1. FRAMING THAT SERVES SOLELY AS A COMPONENT OF NON-BEARING STUD WALL OR CURTAINWALL ASSEMBLY, INCLUDING JAMB FRAMING AT ROLL-DOWN DOORS, JAMB AND/OR HEAD REINFORCEMENT AT OTHER STUD AND CURTAIN ASSEMBLIES, CANTILEVER FRAMING FOR SILLS AND PARTIAL HEIGHT WALLS.
2. ORNAMENTAL METAL, INCLUDING RAILINGS, SUN CONTROL DEVICES AND TRELLISES.

DESIGN CRITERIA

- A. APPLICABLE CODE: PART 2, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CALIFORNIA BUILDING CODE, CBC) 2010 EDITION. SEE S-003 FOR ADDITIONAL CODE REQUIREMENTS FOR NON-STRUCTURAL ITEMS.
B. LIVE LOADS: SEE S-004
C. VIBRATION CRITERIA: 8,000 MICRO-INCHES/SEC DUE TO 185# WALKER AT 100 STEPS/MIN
D. WIND DESIGN PRESSURES: WIND PRESSURES BELOW HAVE BEEN DETERMINED BASED ON REPORT BY CPP, INC. ENTITLED "CLADDING REPORT WIND LOAD ASSESSMENT FOR UCSF MISSION BAY BLOCK 25A, SAN FRANCISCO, CA", DATED MARCH 1, 2013.

Table with 2 columns: ZONE PER ASCE 7-05 FIGURE 6-17, CLOSED BUILDING, NEGATIVE PRESSURES @ 10 SF AREA (PSF). Rows include ZONE 2 (ROOF EDGE) -51, ZONE 3 (ROOF CORNER) -69, ZONE 4 (WALL INTERIOR) -22, ZONE 5 (WALL CORNER) -40.

SEISMIC DESIGN PARAMETERS FOR STRENGTH CHECKS:

OCCUPANCY CATEGORY: III
SEISMIC DESIGN CATEGORY: D
IMPORTANCE FACTOR: I = 1.25
LONGITUDE: -122.590 WEST
LATITUDE: 37.7672 NORTH
SITE CLASS: D

SITE SPECIFIC SPECTRUMS:

Table with 3 columns: PERIOD (SEC), CBC 2010 (LIFE SAFETY), HAYWARD-RODGERS CREEK 84TH PERCENTILE (IMMEDIATE OCCUPANCY). Rows range from 0.01 to 4.00 seconds.

CBC 2010 SPECTRUM HAS BEEN USED FOR DESIGN OF THE MAIN SEISMIC LATERAL FORCE RESISTING SYSTEM. THE HAYWARD-RODGERS CREEK 84TH PERCENTILE SPECTRUM HAS BEEN USED FOR DESIGN OF ALL NON-STRUCTURAL COMPONENTS AND EQUIPMENT IN COMBINATION WITH THE ACCEPTANCE CRITERIA OF ASCE 41 (N-B) IMMEDIATE OCCUPANCY.

STRUCTURAL RESPONSE MODIFICATION FACTORS:

R = 6 (SPECIAL CONCRETE SHEAR WALL)
Cd = 2.5
Cd = 5
Sds = 0.911
PENTHOUSE (OC8F): Rp = 3.5, ap = 2.5, Ip = 1.0

DESIGN BASE SHEAR: V = 0.16W

- F. DESIGN INTERSTORY SEISMIC DRIFT: THE DESIGN INTERSTORY DRIFT BETWEEN A LEVEL AND THE FLOOR BELOW IS AS SHOWN IN THE TABLE BELOW, WHERE H IS THE STORY HEIGHT. ALL NON-STRUCTURAL ITEMS SHALL BE DESIGNED TO ACCOMMODATE THE CBC 2010 DESIGN STORY DRIFT TOGETHER WITH THE CBC ACCEPTANCE CRITERIA AS WELL AS THE HAYWARD-RODGERS CREEK 84TH PERCENTILE DESIGN STORY DRIFT TOGETHER WITH THE ASCE 41 (N-B) IMMEDIATE OCCUPANCY ACCEPTANCE CRITERIA.

Table with 4 columns: FLOOR, STORY HEIGHT, LIFE SAFETY (CBC 2010), IMMEDIATE OCCUPANCY (HAYWARD-RODGERS CREEK 84TH PERCENTILE). Rows include LEVEL 2 through LEVEL 8.

PERIMETER DEFLECTION

- 1. UNDER SUPERIMPOSED DEAD LOAD AND DESIGN LIVE LOADS, THE PERIMETER DEFLECTION IS A MAXIMUM OF 1/2".

FOUNDATION DESIGN CRITERIA:

- A. THE FOUNDATION DESIGN IS BASED ON CRITERIA AND RECOMMENDATIONS PRESENTED IN THE FOLLOWING REPORTS:
1. "GEOTECHNICAL MEMO SUPPORTING DESIGN DEVELOPMENT, UCSF MISSION BAY BUILDING 25A, SAN FRANCISCO, CALIFORNIA" DATED FEBRUARY 1, 2013 BY AMEC.
2. "GEOTECHNICAL REPORT, DESIGN-BUILD OF BLOCK 25A ACADEMIC BUILDING, UNIVERSITY OF CALIFORNIA, SAN FRANCISCO, MISSION BAY CAMPUS," DATED FEBRUARY 2013 BY AMEC.
3. "ADDENDUM 1 TO GEOTECHNICAL REPORT EVALUATION OF SITE SPECIFIC Sds VALUE, DESIGN-BUILD OF BLOCK 25A ACADEMIC BUILDING, UNIVERSITY OF CALIFORNIA, SAN FRANCISCO, MISSION BAY CAMPUS," DATED FEBRUARY 26, 2013.
4. "ADDENDUM 2 TO GEOTECHNICAL REPORT: EVALUATION OF HAYWARD-RODGERS CREEK SCENARIO SPECTRUM, DESIGN-BUILD OF BLOCK 25A ACADEMIC BUILDING, UNIVERSITY OF CALIFORNIA, SAN FRANCISCO, MISSION BAY CAMPUS," DATED FEBRUARY 27, 2013.

- B. AUGER CAST DISPLACEMENT PILES: SEE S-302 FOR ASSUMED PILE CAPACITIES

CONCRETE

REINFORCING STEEL

- 1. ALL BARS, U.O.N.: ASTM A615, GR 60 OR ASTM A706, DEFORMED. ASTM A706 IN ALL CONCRETE WALLS, COLUMNS, BEAMS.
2. BARS TO BE WELDED: ASTM A706.
3. SPIRAL: ASTM A706, DEFORMED.
a. AT SPLICE, LAP 40 DIAMETERS AND PROVIDE SEISMIC HOOK AT ENDS INTO CORE.
b. AT ENDS, PROVIDE TWO FINISHING TURNS AND SEISMIC HOOK AT END INTO CORE.
4. HEADED BARS:
a. WHERE HEADED BARS ARE SHOWN, CONFORM TO REQUIREMENTS OF ACI 318-08 SECTIONS 3.5.9 AND 12.6.
b. HEADED BARS OR LBO APPROVED MECHANICAL "TERMINATORS" WILL BE PERMITTED TO BE SUBSTITUTED FOR HOOKS AT OTHER LOCATIONS AT SUBCONTRACTORS OPTION, SUBJECT TO APPROVAL OF OWNER'S REPRESENTATIVE.

- B. MECHANICAL BAR SPLICES: COVER AND CLEARANCE REQUIREMENTS SHALL BE MAINTAINED BAR COUPLERS.
1. WHERE MECHANICAL SPLICES ARE SHOWN, PROVIDE TYPE 2 COUPLERS U.O.N.
2. MECHANICAL SPLICES WILL BE PERMITTED AT OTHER LOCATIONS AT CONTRACTOR'S OPTION, SUBJECT TO APPROVAL OF OWNER'S REPRESENTATIVE FOR LOCATION AND TYPE OF COUPLER.

- C. CONCRETE MIXES: SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS WHERE CONCRETE IS EXPOSED TO VIEW.
MIX ID USE STRENGTH, Fc WEIGHT

Table with 3 columns: LETTER, DESCRIPTION, PROPERTY. Rows include A. PILE CAPS AND GRADE BEAMS (5,000 PSI), B. SLAB ON GROUND, FORMED SLABS, BEAMS (5,000 PSI), C. WALLS, COLUMNS, (4,000 PSI), D. ARCH WALLS, STAIRS (4,000 PSI), E. COLUMNS AS NOTED (6,000 PSI), F. ARCH COLUMNS AS NOTED (6,000 PSI), G. CURBS, TOPPING SLABS AND EQUIPMENT PADS (4,000 PSI), H. STAIR FILLS (4,000 PSI), I. RAT SLAB (1,500 PSI), J. PROTECTION SLAB (3,000 PSI), K. LIGHTWEIGHT TOPPING SLAB (4,000 PSI).

SHOTCRETE

- A. NOT USED

DRILLED DOWELS

- A. HIT R500-SD ADHESIVE ANCHORING SYSTEM, HILTI, INC. (ICC ESR-2322) OR SET-XP EPOXY ADHESIVE, SIMPSON STRONG-TIE CO. (ICC ESR-2508).

STRUCTURAL STEEL

SHAPES AND PLATES

- 1. W-SHAPES: ASTM A992, U.O.N.
2. ANGLES, CHANNELS, BENT PLATES: ASTM A36, U.O.N.
3. PLATE AND FLAT BARS: ASTM A572, GRADE 50 TYPICAL; ASTM A36 WHERE NOTED.
4. ROUND, SQUARE, AND RECTANGULAR TUBES: ASTM A500, GRADE B.

BOLTS AND RODS

- 1. HIGH STRENGTH BOLTS (H.S. BOLTS): ASTM A325, TYPE 1 OR ASTM F1852, TYPE 1 "TWIST-OFF", U.O.N. ASTM A490 TYPE 1 WHERE SPECIFICALLY NOTED.
2. MACHINE BOLTS AND THRU-BOLTS: ASTM A307.
3. STANDARD ANCHOR BOLTS: ASTM F1554, GRADE 36, U.O.N.
4. HIGH STRENGTH ANCHOR BOLTS: ASTM F1554, GRADE 105.
5. THREADED RODS: ASTM A36 U.O.N.

DEFORMED BAR ANCHORS (DBA): ASTM A496 GR 70.

WELDING

- 1. ELECTRODES: E70XX
2. WELDS USED FOR CONNECTIONS IN THE SEISMIC LOAD RESISTING SYSTEM: SEE SPECIFICATIONS FOR REQUIREMENTS EXCEPT AS SPECIFICALLY NOTED ON DRAWINGS, ALL WELDS AT THE FOLLOWING LOCATIONS SHALL BE CONSIDERED WELDS PART OF THE SEISMIC LOAD RESISTING SYSTEM:
a. BRACED FRAME COLUMN BASE PLATE CONNECTIONS.
b. BRACE CONNECTIONS, INCLUDING GUSSETS.
E. EXPANSION OR WEDGE ANCHORS: HILTI KWIK BOLT TZ EXPANSION ANCHOR, OR APPROVED EQUAL
F. U.O.N. STEEL ELEMENTS AT EXTERIOR LOCATIONS ARE HOT DIPPED GALVANIZED; SEE SPECIFICATIONS FOR PRIMER REQUIREMENTS. PROVIDE GALVANIZING RELIEF HOLES AS REQUIRED, AND FILL WITH FREEZE PLUGS.

STRUCTURAL STEEL FRAMING NOTES

- A. CODE: COMPLY WITH ANSI / AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" 2010, FOR ALL TOLERANCES, SPACINGS, MINIMUM WELD SIZES AND OTHER DETAILS NOTED OR SHOWN.
B. CONNECTIONS: USE CONNECTIONS DESIGNATED AS "TYPICAL" WHERE SPECIFIC CONNECTION DETAILS ARE NOT CALLED OUT. WHERE GEOMETRY OR OTHER CONDITIONS VARY FROM CONDITIONS OF THE TYPICAL CONNECTIONS, PROVIDE SIMILAR CONNECTIONS OF EQUAL STRENGTH.

METAL DECK

- A. ROOF DECK: ASTM A653, SS, GRADE 33, GALVANIZED, MINIMUM YIELD 38 KSI.

HAZARDOUS MATERIALS

- A. RUTHERFORD & CHEKENE ASSUMES NO RESPONSIBILITY FOR THE MANAGEMENT OF HAZARDOUS MATERIALS THAT MAY BE ON THE SITE.
B. RUTHERFORD & CHEKENE HAS NOT PERFORMED INVESTIGATIONS TO DETERMINE THE PRESENCE OF HAZARDOUS MATERIALS. THE DESIGN BUILDER WILL PROVIDE THE RESULTS OF SUCH INVESTIGATIONS IF THEY HAVE BEEN PERFORMED.
C. THE SUBCONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT PERSONNEL WITHIN THE WORK AREA ARE PROTECTED FROM EXPOSURE TO HAZARDOUS MATERIALS. IF HAZARDOUS MATERIALS ARE DISCOVERED, THE SUBCONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTOR AND CEASE WORK UNTIL CONDITIONS CAN BE MAINTAINED IN COMPLIANCE WITH ALL APPLICABLE REGULATIONS.

STRUCTURAL TESTING AND INSPECTION

- A. THE FOLLOWING CHECKLIST IS TO ASSIST THE SUBCONTRACTOR IN SCHEDULING OF TESTING AND INSPECTION RELATED TO STRUCTURAL FEATURES. THE WORK OF OTHER DISCIPLINES MAY REQUIRE TESTING AND INSPECTION THAT IS ADDITIONAL TO THE ITEMS LISTED BELOW.
B. REFER TO APPLICABLE PROVISIONS OF DIVISION 1 OF THE PROJECT SPECIFICATIONS AND THE GENERAL CONDITIONS OF THE CONTRACT FOR DESIGN BUILDER, UNIVERSITY'S TESTING LABORATORY, GEOTECHNICAL ENGINEER AND SUBCONTRACTOR'S RESPONSIBILITIES REGARDING TESTING AND INSPECTION.
D. REFER TO PROJECT SPECIFICATIONS FOR MORE DETAILED REQUIREMENTS FOR TESTS AND INSPECTIONS. THE PROJECT SPECIFICATIONS SHALL TAKE PRECEDENCE OVER THE CHECKLIST.
E. SPECIAL INSPECTORS SHALL BE QUALIFIED BY TRAINING AND EXPERIENCE FOR THE REQUIRED INSPECTIONS AND MUST BE ACCEPTABLE TO THE DESIGN BUILDER. INSPECTORS SHALL THOROUGHLY REVIEW THE APPLICABLE PORTIONS OF THE DOCUMENTS. INSPECTORS SHALL PERFORM ALL DUTIES AND RESPONSIBILITIES AS REQUIRED BY CBC SECTION 1704.

STRUCTURAL OBSERVATION

- A. THE STRUCTURAL ENGINEER WILL GENERALLY REVIEW THE PROGRESS OF THE WORK IN ACCORDANCE WITH SECTION 1702 OF THE CBC, BUT THIS REVIEW SHALL NOT BE CONSTRUED AS SPECIAL INSPECTION.

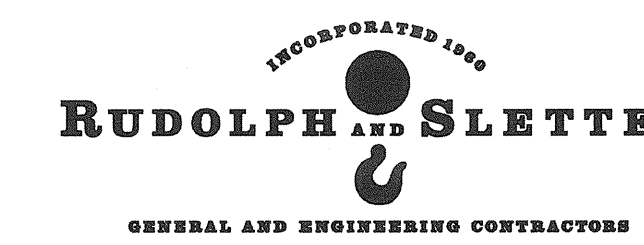
ITEMS NOT INCLUDED IN THIS PACKAGE

- A. THE FOLLOWING ITEMS ARE NOT INCLUDED IN THIS PACKAGE, AND WILL BE INCLUDED IN PACKAGES 2 AND 3:
1. DESIGN BUILD STEEL STAIRS, AND EMBEDMENTS FOR THEM
2. CAST-IN-PLACE CONCRETE STAIRS
3. SITE CONCRETE AND FOUNDATIONS.

Main table with columns: MATERIAL VERIFICATION, TEST OR SPECIAL INSPECTION, TYPE, PERFORMED BY, CODE REFERENCE AND NOTES. Rows include SOILS, PILE FOUNDATIONS, CAST IN PLACE CONCRETE, STRUCTURAL STEEL, METAL DECK, HAZARDOUS MATERIALS, STRUCTURAL TESTING AND INSPECTION, STRUCTURAL OBSERVATION, and ITEMS NOT INCLUDED IN THIS PACKAGE.

WRNS STUDIO

501 SECOND STREET
4TH FLOOR, STE 4022
SAN FRANCISCO
CALIFORNIA 94107
415 489 2224 TEL
415 358 9100 FAX
WWW.WRNSSTUDIO.COM



Structural | Geotechnical Engineers
55 Second Street Suite 600
San Francisco CA 94105
T 415 568 4400
F 415 618 0684
www.ruthchek.com

Table with 2 columns: ISSUES, DATE. Row: PACKAGE 1: EXTERIOR CLADDING AND STRUCTURE RECORD DOCUMENTS, 02/09/15.

Table with 2 columns: REVISION LIST, DATE. Row: 1. BACKCHECK 1, 03/06/15.



UNIVERSITY OF CALIFORNIA
SAN FRANCISCO
FIRE MARSHAL
CDF-OFFICE OF STATE FIRE MARSHAL
APPROVED: 5/22/15
Approval of this plan does not authorize or approve any omission or deviation from applicable regulations. First approval is subject to field inspection. One set of approved plans shall be available on the project site at all times.

UCSF BLOCK 25A
MISSION BAY CAMPUS, SAN FRANCISCO, CA
CONTRACT NO. D80016 | PROJECT NO. M1623

KEYPLAN

PROJECT NO.: 12000.00
DATE: 11/05/2014
SCALE: As indicated

SHEET TITLE:

GENERAL NOTES

SHEET NO.: S-002

2/4/2015 2:04:31 PM