



**FORM 1**  
**CERTIFICATE OF SEISMIC PERFORMANCE LEVEL**

- UC-Designed & Constructed Facility  
 Campus-Acquired or Leased Facility

**BUILDING DATA**

Building Name: 1486-88 5th Ave.  
Address: 1486-88 5th Avenue, San Francisco  
Site location coordinates: Latitude 37.76080 Longitudinal -122.46162

**UCOP SEISMIC PERFORMANCE LEVEL (OR "RATING"): III**

ASCE 41-17 Model Building Type:

- a. Longitudinal Direction: W1: Wood Frame with Wood Shear Panels
- b. Transverse Direction: W1: Wood Frame with Wood Shear Panels

Gross Square Footage: 2725 sf  
Number of stories *above* grade: 3  
Number of basement stories *below* grade: 0

Year Original Building was Constructed: 1915  
Original Building Design Code & Year: NA  
Retrofit Building Design Code & Code (if applicable): 2010 CBC & ASCE 7-05

**SITE INFORMATION**

Site Class: C Basis: UCSF Group 3 Buildings – Geotechnical Assessment, Egan (2019)  
Geologic Hazards:  
Fault Rupture: No Basis: UCSF Group 3 Buildings – Geotechnical Assessment, Egan (2019)  
Liquefaction: No Basis: UCSF Group 3 Buildings – Geotechnical Assessment, Egan (2019)  
Landslide: No Basis: UCSF Group 3 Buildings – Geotechnical Assessment, Egan (2019)

**ATTACHMENT**

Original Structural Drawings: NA  
Seismic Evaluation: NA  
Retrofit Structural Drawings: 1486 & 1488 Fifth Avenue Housing Remodel, UCSF Project M0549, by FTF Engineers dated 9/13/2011 (9 sheets); Sheet S-1.0 and A1 attached.



## CERTIFICATION & PRESUMPTIVE RATING VERIFICATION STATEMENT

I, [Maryann T. Phipps](#), a California-licensed structural engineer, am responsible for the completion of this certificate, and I have no ownership interest in the property identified above. My scope of review to support the completion of this certificate included both of the following ("No" responses must include an explanation):

- a) the review of structural drawings indicating that they are as-built or record drawings, or that they otherwise are the basis for the construction of the building:  Yes  No
- b) visiting the building to verify the observable existing conditions are reasonably consistent with those shown on the structural drawings:  Yes  No

Based on my review, I have verified that the UCOP Seismic Performance Level (SPL) is presumptively permitted by the following UC Seismic Program Guidebook provision (choose one of the following):

- 1) Contract documents indicate that the original design and construction of the aforementioned building is in accordance with the benchmark design code year (or later) building code seismic design provisions for UBC or IBC listed in Table 1 below.
- 2) The existing SPL rating is based on an acceptable basis of seismic evaluation completed in 2006 or later.
- 3) Contract documents indicate that a comprehensive<sup>1</sup> building seismic retrofit design was fully-constructed with an engineered design based on the 1997 UBC/1998 **or later** CBC, and (choose one of the following):
  - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1E (or BSE-R) and BSE-2E (or BSE-C) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 CBC **or later** for EXISTING buildings, and is presumptively assigned an SPL rating of IV.
  - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1 (or BSE-1N) and BSE-2 (or BSE-2N) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 **or later** CBC for NEW buildings, and is presumptively assigned an SPL rating of III.
  - the retrofit project was not completed by the UC campus following UC policies, and is presumptively assigned an SPL rating of IV.

<sup>1</sup> A comprehensive retrofit addresses the entire building structural system as indicated by the associated seismic evaluation, as opposed to addressing selective portions of the structural system.

Campus: UCSF Parnassus  
Building Name: 1486-88 5<sup>th</sup> Ave.  
CAAN ID: 2060  
Auxiliary Building ID: NA



UNIVERSITY  
OF  
CALIFORNIA

Date: 2/3/2020

**CERTIFICATION SIGNATURE**

Maryann T. Phipps  
Print Name

President  
Title

S2995  
CA Professional Registration No.

6/30/2020  
License Expiration Date

*Maryann T. Phipps*  
Signature

2/3/2020  
Date

AFFIX SEAL HERE



Estructure, (510) 235-3116, 1144 65th St Suite A, Oakland  
Firm Name, Phone Number, and Address



**Table 1: Benchmark Building Codes and Standards**

Building Type <sup>a,b</sup>	Building Seismic Design Provisions	
	UBC	IBC
Wood frame, wood shear panels (Types W1 and W2)	1976	2000
Wood frame, wood shear panels (Type W1a)	1976	2000
Steel moment-resisting frame (Types S1 and S1a)	1997	2000
Steel concentrically braced frame (Types S2 and S2a)	1997	2000
Steel eccentrically braced frame (Types S2 and S2a)	1988 <sup>g</sup>	2000
Buckling-restrained braced frame (Types S2 and S2a)	<sup>f</sup>	2006
Metal building frames (Type S3)	<sup>f</sup>	2000
Steel frame with concrete shear walls (Type S4)	1994	2000
Steel frame with URM infill (Types S5 and S5a)	<sup>f</sup>	2000
Steel plate shear wall (Type S6)	<sup>f</sup>	2006
Cold-formed steel light-frame construction—shear wall system (Type CFS1)	1997 <sup>h</sup>	2000
Cold-formed steel light-frame construction—strap-braced wall system (Type CFS2)	<sup>f</sup>	2003
Reinforced concrete moment-resisting frame (Type C1) <sup>i</sup>	1994	2000
Reinforced concrete shear walls (Types C2 and C2a)	1994	2000
Concrete frame with URM infill (Types C3 and C3a)	<sup>f</sup>	<sup>f</sup>
Tilt-up concrete (Types PC1 and PC1a)	1997	2000
Precast concrete frame (Types PC2 and PC2a)	<sup>f</sup>	2000
Reinforced masonry (Type RM1)	1997	2000
Reinforced masonry (Type RM2)	1994	2000
Unreinforced masonry (Type URM)	<sup>f</sup>	<sup>f</sup>
Unreinforced masonry (Type URMa)	<sup>f</sup>	<sup>f</sup>
Seismic isolation or passive dissipation	1991	2000

Note: This table has been adapted from ASCE 41-17 Table 3-2. Benchmark Building Codes and Standards for Life Safety Structural Performed at BSE-1E.

Note: UBC = Uniform Building Code. IBC = International Building Code.

<sup>a</sup> Building type refers to one of the common building types defined in Table 3-1 of ASCE 41-17.

<sup>b</sup> Buildings on hillside sites shall not be considered Benchmark Buildings.

<sup>c</sup> not used

<sup>d</sup> not used

<sup>e</sup> not used

<sup>f</sup> No benchmark year; buildings shall be evaluated in accordance with Section III.J.

<sup>g</sup> Steel eccentrically braced frames with links adjacent to columns shall comply with the 1994 UBC Emergency Provisions, published September/October 1994, or subsequent requirements.

<sup>h</sup> Cold-formed steel shear walls with wood structural panels only.

<sup>i</sup> Flat slab concrete moment frames shall not be considered Benchmark Buildings.

STRUCTURAL NOTES:

1. DESIGN CRITERIA:

- 1.1 NOT USED.
1.2 DEAD LOADS: BASED ON WEIGHTS OF EXISTING AND NEW MATERIALS OF CONSTRUCTION.
1.3 LIVE LOADS:
ROOF (FLAT) = 20 PSF
FLOOR = 40 PSF

1.4 SEISMIC (ASCE 7-05):
V = 1.00 W (LRFD)
SEISMIC DESIGN CATEGORY E
IMPORTANCE FACTOR J = 1.0
REDUNDANCY FACTOR RHO = 1.3
SITE CLASS D
(LAT, LONG) (37.7608, -122.4618)
MAPPED VALUES Ss = 1.651
Sv = 1.257
SEISMIC VALUES Sps = 1.10
S1 = 0.838
STRUCTURAL SYSTEM FACTOR R = 6.5 PLYWOOD SW ANALYSIS PROCEDURE LINEAR STATIC

1.5 WIND LOADS (ASCE 7-05 SIMPLIFIED PROCEDURE):

- PRIMARY SYSTEMS
P = lambda Kzt I p330 = 12.9 PSF
WHERE,
V = 85 MPH
lambda = 1.0
Kzt = 1.0
p330 = 12.9 PSF
Iw = 1.0
BASIC WIND SPEED H = 40FT, EXPOSURE B
TOPOGRAPHIC EFFECT
PRIMARY SYSTEMS, ZONE A
STANDARD OCCUPANCY

2. STRUCTURAL DRAWINGS:

- 2.1 NOTES, TYPICAL DETAILS AND SCHEDULES APPLY TO ALL STRUCTURAL WORK UNLESS NOTED OTHERWISE.
2.2 REVIEW ALL EXISTING FEATURES AND CONDITIONS UPON WHICH THESE DRAWINGS RELY.
2.3 COMPARE STRUCTURAL DRAWINGS WITH THE VARIOUS OTHER DRAWINGS AND SPECIFICATIONS BEFORE COMMENCING THE WORK.
2.4 DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONAL INFORMATION.
2.5 SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND SPECIFICATIONS FOR INSERTS, SLEEVES, BLOCKOUTS AND OTHER CONDITIONS.
2.6 SEE ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING AND DAMPROOFING DETAILS.

3. CONSTRUCTION:

- 3.1 NOT USED.
3.2 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION OF THIS BUILDING.
3.3 THE CONTRACTOR SHALL TAKE ALL PRECAUTIONS NECESSARY TO PREVENT DAMAGE AND MINIMIZE MOVEMENT/SETTLEMENT OF EXISTING OR NEW CONSTRUCTION INSIDE OR OUTSIDE OF THE PROJECT LIMITS.
3.4 THE CONTRACTOR'S TEMPORARY MEASURES SHALL BE ARRANGED OR DESIGNED SO AS TO NOT ALTER OR AFFECT THE PERMANENT STRUCTURE.
3.5 THE IMPOSED CONSTRUCTION LOADS SHALL NOT BE MORE THAN DESIGN LIVE LOADS.
3.6 WORK SHALL INCLUDE REPAIR AND/OR REPLACEMENT OF DEFECTIVE ITEMS.
3.7 OPENINGS IN FLOORS, SHEAR WALLS, BEAMS, OR JOISTS LARGER THAN THOSE SHOWN ON TYPICAL DETAILS OR STRUCTURAL DRAWINGS SHALL BE REVIEWED BY UNIVERSITY'S REPRESENTATIVE BEFORE PROCEEDING WITH THE WORK.

4. EXISTING CONDITIONS:

- 4.1 INFORMATION REGARDING EXISTING CONDITIONS IS PRESENTED FOR REFERENCE ONLY.
4.2 THE REMOVAL CUTTING, DRILLING, ETC. OF EXISTING WORK SHALL BE PERFORMED WITH GREAT CARE AND SMALL TOOLS IN ORDER NOT TO JEOPARDIZE THE STRUCTURAL INTEGRITY OF THE BUILDING.

5. EXCAVATION, UNDERPINNING AND SHORING

- 5.1 THE CONTRACTOR SHALL PROVIDE ALL MEASURES AND PRECAUTIONS NECESSARY TO PREVENT DAMAGE AND MINIMIZE SETTLEMENT OF EXISTING OR NEW CONSTRUCTION INSIDE OR OUTSIDE OF THE PROJECT LIMITS.
ANY DAMAGE TO NEW OR EXISTING CONSTRUCTION INSIDE OR OUTSIDE OF THE PROJECT LIMITS CAUSED BY CONSTRUCTION TECHNIQUES OR MOVEMENTS OF THE SOIL RETENTION SYSTEMS IS THE RESPONSIBILITY OF THE CONTRACTOR.

- 5.2 DESIGN AND CONSTRUCTION OF TEMPORARY AND/OR PERMANENT UNDERPINNING, SHORING AND BULK HEADING FOR EARTH RETENTION DURING EXCAVATION SHALL BE BY AN EXPERIENCED SUBCONTRACTOR WHO SPECIALIZES IN THIS TYPE OF WORK.
5.3 SHORING, UNDERPINNING, AND EARTH RETENTION CALCULATIONS AND DRAWINGS, IF REQUIRED, SHALL BE PREPARED AND SUBMITTED TO THE UNIVERSITY'S REPRESENTATIVE FOR REVIEW PRIOR TO CONSTRUCTION.
5.4 PRIOR TO ANY EXCAVATION, THE CONTRACTOR SHALL ESTABLISH BENCH MARKS AROUND THE PERIMETER OF THE AREA TO BE EXCAVATED.
5.5 THE CONTRACTOR SHALL PROVIDE POSITIVE PROTECTION (MAT/SHEET COVERINGS) FOR ALL EXCAVATION SLOPES TO PROTECT SLOPES FROM INSTABILITY AND DETERIORATION DUE TO RAIN OR WIND.
5.6 THE UNIVERSITY'S SOIL TESTING LABORATORY SHALL REVIEW AND MONITOR THE EXCAVATION AND FOUNDATION/RETAINING WALL CONSTRUCTION.

6. FOUNDATIONS/SITE PREPARATION:

- 6.1 FOUNDATION DESIGN IS BASED ON CHAPTER 18, CBC.
CONTINUOUS FOOTINGS:
MAXIMUM SOIL PRESSURE = 1,500 PSF DL + LL
= 2,000 PSF DL + LL + SEISMIC/WIND
ISOLATED FOOTINGS:
MAXIMUM SOIL PRESSURE = 1,500 PSF DL + LL
= 2,000 PSF DL + LL + EQ
RETAINING WALLS:
EFP = 45 PCF (DRAINED)
SURCHARGE RATE = .03
LATERAL BEARING = N/A SLAB RESISTS SLIDING
FRICTION = N/A SLAB RESISTS SLIDING

- 6.2 FOOTINGS SHALL EXTEND TO SUCH DEPTH AS TO BEAR ON FIRM, UNDISTRIBUTED SOIL.
6.3 MATERIALS FOR SUB-CAPILLARY BREAK UNDER CONCRETE SLABS ON GRADE SHALL BE FREE-DRAINING GRAVEL OR CRUSHED ROCK.
6.4 BEFORE BACKFILLING BEHIND CONCRETE WALLS (BASEMENT WALLS, RETAINING WALLS, ETC.) CONCRETE SHALL HAVE ATTAINED FULL DESIGN STRENGTH AND ALL SUPPORTS (FLOORS, SLABS, BEAMS, ETC.) WHICH ARE REQUIRED FOR THE STABILITY OF THE WALL SHALL HAVE BEEN COMPLETED.
6.5 FOOTING EXCAVATIONS SHALL BE CLEANED OF LOOSE SOILS.
6.6 SEE ARCHITECTURAL, ELECTRICAL, HVAC, PLUMBING AND MECHANICAL DRAWINGS FOR DETAILS AT DOOR AND WINDOW OPENINGS, FLOOR TYPE HINGES, TYPE AND LOCATION OF ALL FLOOR FINISHES, FLOOR DEPRESSIONS AND CURBS, ETC.
6.7 CONCRETE SHALL BE READY MIXED CONFORMING TO ASTM C94, HAVING THE FOLLOWING MINIMUM 28 DAY ULTIMATE COMPRESSIVE STRENGTHS AND UNIT WEIGHTS:
FOOTINGS, RETAINING WALLS 3,000 PSI, 145 PCF
GRADE BEAMS 3,000 PSI, 145 PCF
SLAB-ON-GRADE, SIDEWALK, CURBS, BACKFILL CONCRETE, ALL OTHER CONCRETE 2,500 PSI, 145 PCF
6.8 USE MINIMUM 4-1/2 SACKS OF CEMENT PER CUBIC YARD FOR 2500 PSI CONCRETE AND 5 SACKS FOR 3000 PSI CONCRETE.

7. CONCRETE WORK:

- 7.1 FORMS SHALL BE PROPERLY CONSTRUCTED CONFORMING TO CONCRETE SURFACES AS SHOWN ON THE DRAWINGS.
7.2 SEE ARCHITECTURAL, ELECTRICAL, HVAC, PLUMBING AND MECHANICAL DRAWINGS FOR DETAILS AT DOOR AND WINDOW OPENINGS, FLOOR TYPE HINGES, TYPE AND LOCATION OF ALL FLOOR FINISHES, FLOOR DEPRESSIONS AND CURBS, ETC.
7.3 PIPES OTHER THAN ELECTRICAL CONDUITS 1 INCH DIAMETER MAXIMUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED BY THE UNIVERSITY'S REPRESENTATIVE.
7.4 ALL REINFORCING, EMBEDMENTS, INSERTS, ETC., SHALL BE POSITIVELY SECURED IN PROPER LOCATION BEFORE CONCRETE IS PLACED.
7.5 WHERE NOT SHOWN ON STRUCTURAL DRAWING, SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS OF SLAB AND WALL OPENINGS, SLAB EDGE LOCATIONS, LOCATIONS OF MISCELLANEOUS INTERIOR CONCRETE WALLS AND CURBS, TOP OF FLOOR SLAB ELEVATIONS, SLAB DEPRESSIONS REQUIRED TO ACCOMMODATE ARCHITECTURAL FLOOR FINISH DETAILS, CONCRETE STAIR LOCATIONS, ETC.
7.6 CONCRETE SHALL BE READY MIXED CONFORMING TO ASTM C94, HAVING THE FOLLOWING MINIMUM 28 DAY ULTIMATE COMPRESSIVE STRENGTHS AND UNIT WEIGHTS:
FOOTINGS, RETAINING WALLS 3,000 PSI, 145 PCF
GRADE BEAMS 3,000 PSI, 145 PCF
SLAB-ON-GRADE, SIDEWALK, CURBS, BACKFILL CONCRETE, ALL OTHER CONCRETE 2,500 PSI, 145 PCF
7.7 USE MINIMUM 4-1/2 SACKS OF CEMENT PER CUBIC YARD FOR 2500 PSI CONCRETE AND 5 SACKS FOR 3000 PSI CONCRETE.

- 7.8 CONTRACTOR SHALL SUBMIT FOR REVIEW BY THE UNIVERSITY'S REPRESENTATIVE THE CONTRACTOR'S PROPOSED CONCRETE MIXES, DESIGNED BY THE CONCRETE SUPPLIER AND REVIEWED BY THE UNIVERSITY'S TESTING AGENCY.
7.9 PROPORTION CONCRETE WITH A MINIMUM OF 20% AND A MAXIMUM OF 50% FLY ASH OR SLAG REPLACEMENT.
7.10 USE WATER THAT IS CLEAN AND FREE FROM INJURIOUS AMOUNTS OF OILS, ACIDS, ALKALIS, SALTS, ORGANIC MATERIALS, OR OTHER SUBSTANCES DELETERIOUS TO CONCRETE OR REINFORCEMENT.
7.11 NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
7.12 SLUMP SHALL BE THE MINIMUM CONSISTENT WITH PROPER PLACING, IN GENERAL:
STRUCTURAL SLABS 2-1/2" TO 3-1/2"
FOOTINGS, SLAB ON GRADE 3-1/2" TO 4-1/2"
THIN OR CONGESTED SECTIONS 4-1/2" TO 5"
7.13 USE 3/4" AGGREGATE WHEREVER CLEARANCES PERMIT. USE 3/8" AGGREGATE ONLY WHERE NECESSARY FOR PROPER PLACING, SUCH AS IN THIN SECTIONS, ETC.
7.14 ALL CONCRETE EXCEPT SLABS ON GRADE 6" THICK OR LESS SHALL BE MECHANICALLY VIBRATED SO AS TO COMPLETELY FILL THE FORMS WITHOUT CAUSING UNDUE SEGREGATION.
7.15 HORIZONTAL CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN ON THE DRAWINGS, AND THE HARDENED CONCRETE SURFACES SHALL BE CLEANED BY SAND-BLASTING OR OTHER APPROVED MEANS TO EXPOSE FIRMLY EMBEDDED AGGREGATES PRIOR TO POURING ADDITIONAL CONCRETE IN CONTACT WITH THESE SURFACES.
7.16 VERTICAL CONSTRUCTION JOINTS SHALL BE FORMED AND KEYED AND NOT OVER 60 FEET APART.
7.17 CONTRACTOR SHALL INFORM THE UNIVERSITY'S REPRESENTATIVE AT LEAST 2 DAYS PRIOR TO POURING ANY STRUCTURAL CONCRETE FOR THE OPPORTUNITY TO REVIEW THE WORK PRIOR TO PLACEMENT.

8. CONCRETE REINFORCING STEEL:

- 8.1 REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60 FOR ALL BAR SIZES AND ASTM A706 GRADE 60 FOR ALL WELDED BARS.
8.2 WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 IN FLAT SHEETS, AND IN SLABS MAY BE RAISED INTO POSITION DURING THE CONCRETE POURING OPERATION.
8.3 ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR AND REINFORCED CONCRETE," ACI 318 AND THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315.
8.4 UNLESS OTHERWISE NOTED, MAINTAIN COVERAGE TO FACE OF BARS AS FOLLOWS:
a. CONCRETE CAST AGAINST EARTH 3"
b. FORMED SURFACES EXPOSED TO EARTH OR WEATHER NO. 5 AND SMALLER, WIRE MESH 1-1/2"
ALL OTHER BARS 2"
c. JOISTS, SUSPENDED SLABS, INTERIOR WALL SURFACES NO. 11 AND SMALLER 3/4"
NO. 14 AND LARGER 1-1/2"
d. COLUMNS, BEAMS 1-1/2"
8.5 REINFORCING SHALL BE CONTINUOUS WITH SPLICES ONLY WHERE SHOWN.
8.6 FOR MINIMUM LAP LENGTH, SEE SCHEDULE UNLESS OTHERWISE NOTED. SPLICES TO BE STAGGERED SO THAT HALF OR LESS OF BARS ARE LAPPED AT ONE POINT.
8.7 BAR SUPPORTS IN CONTACT WITH EXPOSED SURFACES SHALL BE PLASTIC TIPPED.
8.8 BEAM AND SLAB REINFORCING SHALL NOT BE SLEEVED OR OTHERWISE INTERRUPTED EXCEPT AS SHOWN ON THE STRUCTURAL DRAWINGS.
8.9 ALL WALLS AND SLABS SHALL BE DOWELED INTO FOOTINGS, WALLS, BEAMS, GIRDERS, COLUMNS OR SLABS WITH BARS OF THE SAME SIZE AND SPACING, UNLESS NOTED OTHERWISE.
8.10 ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS.

9. STRUCTURAL STEEL:

- 9.1 MISCELLANEOUS IRON AND STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS," LATEST EDITION, AND THE "CODE FOR STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES," LATEST EDITION.
9.2 ALL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER UP. PROVIDE CAMBERS AS INDICATED ON THE DRAWINGS.
9.3 AFTER FABRICATION, ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS AND A COAT OF PRIMER PAINT APPLIED.
9.4 THE CONTRACTOR WILL BE RESPONSIBLE FOR REVIEWING ALL STRUCTURAL STEEL DETAILS, WELDING SEQUENCES, AND FABRICATION AND ERECTION PROCEDURES WITH STEEL MANUFACTURER, FOR THE INTENDED USE OF STRUCTURAL STEEL.
9.5 THE CONTRACTOR SHALL SUBMIT TO THE UNIVERSITY'S REPRESENTATIVE, FOR REVIEW, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS AND ERECTION DIAGRAMS FOR ALL STRUCTURAL STEEL.
9.6 THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY LOADING CONDITIONS DURING CONSTRUCTION AND SHALL PROVIDE BRACING AND SHORING WHERE REQUIRED.
9.7 THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE UNIVERSITY'S REPRESENTATIVE.

- 9.8 ALL ADDITIONAL STEEL REQUIRED BY THE CONTRACTOR FOR ERECTION PURPOSES AND SITE ACCESS OF STOCKPILED MATERIALS SHALL BE PROVIDED AT NO COST TO THE UNIVERSITY.
9.9 EXCEPT WHERE OTHERWISE SHOWN, STEEL SECTIONS SHALL CONFORM TO THE FOLLOWING:
a. ALL STRUCTURAL STEEL PLATES, BARS, ETC. ASTM A572, GRADE 50
b. WIDE FLANGE TYPICAL BEAMS AND GIRDERS ASTM A992, GRADE 50
c. STRUCTURAL TUBES (RECTANGULAR OR SQUARE HSS) ASTM A500 GRADE B (Fy = 46ksi)
d. STRUCTURAL PIPES (ROUND HSS) ASTM A500 GRADE B (Fy = 42ksi)
e. TYPICAL (GRAVITY) BASE PLATES ASTM A572, GRADE 50
f. ALL CONTINUITY, REINFORCING, AND SHEAR PLATES ASTM A572, GRADE 50
g. GUSSET PLATES, BARS AND BASE PLATES ASSOCIATED W/ MOMENT AND BRACED FRAMES ASTM A572, GRADE 50
h. ANCHOR BOLTS (A.B.) A36 U.N.O.
i. MACHINE BOLTS (M.B.) A307
j. HIGH STRENGTH BOLTS (H.S.B.) A325X-SC, U.N.O.
9.10 WELDING OF STRUCTURAL STEEL:
a. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF AWS CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION, LATEST EDITION, AND SHALL BE PERFORMED BY WELDERS CERTIFIED IN THE APPLICABLE PROCEDURE & POSITION.
b. E-70XX ELECTRODES SHALL BE USED AT ALL WELDED STEEL CONNECTIONS.
c. ALL BUTT WELDING SHALL BE FULL PENETRATION WELDS UNLESS OTHERWISE NOTED.
d. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) THAT HAS BEEN REVIEWED BY THE ENGINEER OF RECORD AND THE TESTING AND INSPECTION AGENCY.
e. ALL WELDS USED IN MEMBERS AND CONNECTIONS IN THE SEISMIC LOAD RESISTING SYSTEM (SLRS) SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 0°F, AS DETERMINED BY AWS CLASSIFICATION OR MANUFACTURER CERTIFICATION.
f. ALL COMPLETE PENETRATION WELDS SHALL BE STARTED AND ENDED ON RUN-OFF TABS WHERE PRACTICAL.
g. WELD DAMS SHALL NOT BE USED.
h. ALL DEFECTIVE WELDS SHALL BE GROUND OUT, REPAIRED, AND RETESTED AT THE CONTRACTOR'S EXPENSE.
i. ALL WELDS SHALL BE STARTED AND ENDED WITH A MINIMUM LENGTH OF ONE INCH ON WELD TABS ("RUN OFF" TABS) EXCEPT AT ACCESS HOLES IN BEAM/GIRDER WEBS.
j. ALL COMPLETE PENETRATION GROOVE WELDS SHALL BE ULTRASONICALLY (UT) EXAMINED FOR THE FULL LENGTH.
k. BACKING BAR REMOVAL AREAS AND FILLET WELDS ON CONTINUITY PLATES SHALL BE EXAMINED FOR THE FULL LENGTH BY THE MAGNETIC PARTICLE TESTING (MPT) METHOD.
l. A COMPLETE WELDING PROCEDURE SHALL BE SUBMITTED TO AND APPROVED BY THE UNIVERSITY'S REPRESENTATIVE BEFORE ANY WELDING IS COMMENCED.

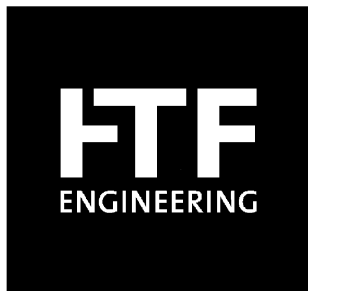
Description of Work: Residential remodel with seismic retrofit.

Index of Drawings:

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S-4.3 - Framing Details IV

UCSF UNIVERSITY OF CALIFORNIA SAN FRANCISCO
Capital Programs
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DESIGN PROFESSIONAL

FIRE MARSHAL

DIVISION OF THE STATE ARCHITECT

DO NOT SCALE DRAWING. ALL MARKED DIMENSIONS TAKE PRECEDENT. ALL DIMENSIONS TO BE VERIFIED IN THE FIELD BY CONTRACTOR PRIOR TO EXECUTION OF WORKS.

Table with 3 columns: NO., DESCRIPTION, DATE. Contains revision history for plan check, response to plan check, and record drawings.

PROJECT TITLE: 1486 & 1488 FIFTH AVE HOUSING REMODEL

DRAWING TITLE: GENERAL NOTES

Table with 2 columns: PROJECT NO./CAAN NO./DRAWN/CHECKED BY/DATE and FTF PROJECT NO./FILE NO./SHEET NO. Includes project number M0549 and sheet number S-1.0.

