



FORM 1
CERTIFICATE OF SEISMIC PERFORMANCE LEVEL

- UC-Designed & Constructed Facility**
 Campus-Acquired or Leased Facility

BUILDING DATA

Building Name: [Aldea San Miguel 11](#)
Address: [90 Behr Ave, San Francisco](#)
Site location coordinates: Latitude [37.7578](#) Longitudinal [-122.4551](#)

UCOP SEISMIC PERFORMANCE LEVEL (OR "RATING"): III

ASCE 41-17 Model Building Type:

- a. Longitudinal Direction: [W1: Wood Frame with Wood Shear Panels](#)
- b. Transverse Direction: [W1: Wood Frame with Wood Shear Panels](#)

Gross Square Footage: [11,592](#)
Number of stories *above* grade: [3](#)
Number of basement stories *below* grade: [0](#)

Year Original Building was Constructed: [1999](#)
Original Building Design Code & Year: [CBC - 1995](#)
Retrofit Building Design Code & Code (if applicable): [NA](#)

SITE INFORMATION

Site Class: [C](#) Basis: ([NA, June 2019, Pg. 1-3](#))
Geologic Hazards:
Fault Rupture: [No](#) Basis: [UCSF Presumptive Buildings – Geotechnical Assessment, Egan \(2019\)](#)
Liquefaction: [No](#) Basis: [UCSF Presumptive Buildings – Geotechnical Assessment, Egan \(2019\)](#)
Landslide: [No](#) Basis: [UCSF Presumptive Buildings – Geotechnical Assessment, Egan \(2019\)](#)

ATTACHMENT

Original Structural Drawings: ([Aldea Housing, Nishkian & Assoc. Inc, 7/25/97, S0.1](#)) or
Seismic Evaluation: [NA](#)
Retrofit Structural Drawings: [NA](#)



CERTIFICATION & PRESUMPTIVE RATING VERIFICATION STATEMENT

I, [Maryann T. Phipps](#), a California-licensed structural engineer, am responsible for the completion of this certificate, and I have no ownership interest in the property identified above. My scope of review to support the completion of this certificate included both of the following ("No" responses must include an explanation):

- a) the review of structural drawings indicating that they are as-built or record drawings, or that they otherwise are the basis for the construction of the building: Yes No
- b) visiting the building to verify the observable existing conditions are reasonably consistent with those shown on the structural drawings: Yes No

Based on my review, I have verified that the UCOP Seismic Performance Level (SPL) is presumptively permitted by the following UC Seismic Program Guidebook provision (choose one of the following):

- 1) Contract documents indicate that the original design and construction of the aforementioned building is in accordance with the benchmark design code year (or later) building code seismic design provisions for UBC or IBC listed in Table 1 below.
- 2) The existing SPL rating is based on an acceptable basis of seismic evaluation completed in 2006 or later.
- 3) Contract documents indicate that a comprehensive¹ building seismic retrofit design was fully-constructed with an engineered design based on the 1997 UBC/1998 **or later** CBC, and (choose one of the following):
 - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1E (or BSE-R) and BSE-2E (or BSE-C) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 CBC **or later** for EXISTING buildings, and is presumptively assigned an SPL rating of IV.
 - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1 (or BSE-1N) and BSE-2 (or BSE-2N) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 **or later** CBC for NEW buildings, and is presumptively assigned an SPL rating of III.
 - the retrofit project was not completed by the UC campus following UC policies, and is presumptively assigned an SPL rating of IV.

¹ A comprehensive retrofit addresses the entire building structural system as indicated by the associated seismic evaluation, as opposed to addressing selective portions of the structural system.

Campus: UCSF
Building Name: Aldea San Miguel 11
CAAN ID: 3016
Auxiliary Building ID: NA



UNIVERSITY
OF
CALIFORNIA

Date: 8/21/2019

CERTIFICATION SIGNATURE

Maryann T. Phipps
Print Name

President
Title

S2995
CA Professional Registration No.

6/30/2020
License Expiration Date

Maryann T. Phipps
Signature

8/21/2019
Date

AFFIX SEAL HERE



9/4/2019

Estructure, (510) 235-3116, 1144 65th St Suite A, Oakland
Firm Name, Phone Number, and Address



Table 1: Benchmark Building Codes and Standards

Building Type ^{a,b}	Building Seismic Design Provisions	
	UBC	IBC
Wood frame, wood shear panels (Types W1 and W2)	1976	2000
Wood frame, wood shear panels (Type W1a)	1976	2000
Steel moment-resisting frame (Types S1 and S1a)	1997	2000
Steel concentrically braced frame (Types S2 and S2a)	1997	2000
Steel eccentrically braced frame (Types S2 and S2a)	1988 ^g	2000
Buckling-restrained braced frame (Types S2 and S2a)	f	2006
Metal building frames (Type S3)	f	2000
Steel frame with concrete shear walls (Type S4)	1994	2000
Steel frame with URM infill (Types S5 and S5a)	f	2000
Steel plate shear wall (Type S6)	f	2006
Cold-formed steel light-frame construction—shear wall system (Type CFS1)	1997 ^h	2000
Cold-formed steel light-frame construction—strap-braced wall system (Type CFS2)	f	2003
Reinforced concrete moment-resisting frame (Type C1) ⁱ	1994	2000
Reinforced concrete shear walls (Types C2 and C2a)	1994	2000
Concrete frame with URM infill (Types C3 and C3a)	f	f
Tilt-up concrete (Types PC1 and PC1a)	1997	2000
Precast concrete frame (Types PC2 and PC2a)	f	2000
Reinforced masonry (Type RM1)	1997	2000
Reinforced masonry (Type RM2)	1994	2000
Unreinforced masonry (Type URM)	f	f
Unreinforced masonry (Type URMa)	f	f
Seismic isolation or passive dissipation	1991	2000

Note: This table has been adapted from ASCE 41-17 Table 3-2. Benchmark Building Codes and Standards for Life Safety Structural Performed at BSE-1E.

Note: UBC = Uniform Building Code. IBC = International Building Code.

^a Building type refers to one of the common building types defined in Table 3-1 of ASCE 41-17.

^b Buildings on hillside sites shall not be considered Benchmark Buildings.

^c not used

^d not used

^e not used

^f No benchmark year; buildings shall be evaluated in accordance with Section III.J.

^g Steel eccentrically braced frames with links adjacent to columns shall comply with the 1994 UBC Emergency Provisions, published September/October 1994, or subsequent requirements.

^h Cold-formed steel shear walls with wood structural panels only.

ⁱ Flat slab concrete moment frames shall not be considered Benchmark Buildings.

ABBREVIATION:

Table of abbreviations and their corresponding full names, including ARCH, BOARD, BLOCKING, B.N., BOT., BRG., B.S., BTWN., CTR.J., OR C.J., C.L.J., CMU, COL., CONC., CONN., CONST., CONT., C.P., C.T.P., DET., DIA., DIM., D.J., DWG., DW'G, D.W., D.O., E.F., EL., E.N., EXIST., OR (E), E.S., E.W.F., EXT., EX.W., FIN.FL., F.O.C.O., F.S., FTG., F.W., F.N., F.P., GA., GALV., G.L., HORIZ., H.S.B., I.D., INT., J.H., L.L.V., L.L.H., MAX., M.B., MECH., MIN., MISC., N.I.C., NO., N.S., N.T.S., NEW OR (N), O.C., O.D., O.H., OPNG., P.D.F., PL., PLY., P.P., P.T., REINF., RWD., S.A.D., S.M.D., S.D.S., SECT., SIM., SO., SPECS., STD., STGD., STIFF., SYMM., T.O.PL., T&B, THD., T.N., T.O.C., TYP., T.O.S., T.O.W., U.O.N., VERT., V.I.F., W.P., W.S.

MANUFACTURED TRUSSES:

- 1. MATERIALS, DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES (TP), LATEST EDITION, THE 1995 CALIFORNIA BUILDING CODE, AND ANY APPLICABLE LOCAL CODES.
2. DESIGN LOADS: AS SHOWN ON SHEET S5.2
3. A COMPLETE SHOP DRAWING SUBMITTAL SHALL BE MADE TO THE ARCHITECT PRIOR TO FABRICATION. FABRICATION SHALL NOT OCCUR UNTIL SHOP DRAWINGS HAVE BEEN RETURNED APPROVED. SUBMITTAL SHALL INCLUDE:
A. COMPLETE LAYOUT PLANS SHOWING DIMENSIONED LOCATIONS OF ALL TRUSSES AND SHOWING ALL REQUIRED BRIDGING, BRACING AND BLOCKING.
B. COMPLETE SET OF DETAILS SHOWING EACH DIFFERENT BEARING CONDITION.
C. SPECIAL INDICATION ON THE LAYOUT PLANS OF ALL TRUSSES AND BLOCKING TO BE USED FOR SHEAR TRANSFER AS PER STRUCTURAL DRAWINGS.
D. COMPLETE SET OF CALCULATIONS FOR ALL TRUSS TYPES INCLUDING DEFINITIONS OF ABBREVIATIONS USED. SUBMITTAL MATERIAL SHALL BE SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA. SUBMITTAL SHALL CONSIST OF TWO SETS OF BLUELINE PRINTS AND ONE SET OF SEPIA REPRODUCIBLE PRINTS. SEPIAS SHALL BE HIGH QUALITY PRINTS WITH ALL LINES CLEARLY READABLE AND LITTLE OR NO BACKROUND.
4. SUBMIT DESIGN AND PLANS TO UCSF-FACILITIES MANAGEMENT CODE REVIEW & INSPECTION DEPARTMENT AT LEAST 4 WEEKS PRIOR TO ERECTION.

STRUCTURAL STEEL:

- 1. STEEL USAGE:
A. ALL ROLLED SHAPES AND PLATES, ASTM A36 OR ASTM A572, GR.50
B. TUBE: ASTM A500, GRADE B.
C. BOLTS: ASTM A307 (M.B.)
D. SHEAR CONNECTORS: ASTM A108
E. ANCHOR BOLTS: ASTM A307 (A108)
F. THREADED RODS: ASTM A36
2. ALL STRUCTURAL STEEL TO BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.
3. ALL WELDING TO CONFORM TO A.W.S. AND TO BE PERFORMED BY CERTIFIED WELDERS.
4. ALL BUTT WELDS ARE TO BE COMPLETE PENETRATION U.O.N. ALL FILLET WELDS TO BE A.I.S.C. MINIMUM SIZES BASED ON THICKNESS OF MATERIAL JOINED, U.O.N.
5. WELDING ELECTRODE - E70XX, U.O.N.
6. BOLT HOLES FOR MACHINE BOLTS SHALL BE NO MORE THAN 1/16" OVERSIZE, U.O.N. WHERE OVERSIZED HOLES REQUIRED, PROVIDE 5/16"X3"X3" PLATE WASHER WELDED TO THE STRUCTURAL MEMBER.
7. ALL STEEL MEMBERS CONNECTING TO OR SUPPORTING WOOD FRAMING SHALL HAVE 1/2" DIAMETER THREADED STUDS AT 24" ON CENTER TYPICAL, U.O.N.
8. CONTRACTORS SHALL SUBMIT DETAILED SHOP DRAWINGS PER SPECIFICATIONS FOR ALL STEEL TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
9. ALL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED, U.O.N.
10. ALL DETAILS ARE TYPICAL. FOR CONDITIONS NOT SPECIFICALLY SHOWN, TESTING AND INSPECTION DEPARTMENT, ARCHITECT, STRUCTURAL ENGINEER AND OWNER.

CONCRETE UNIT MASONRY (CMU):

- 1. CONCRETE BLOCK SHALL BE HOLLOW, LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C90, GRADE N111. MORTAR SHALL BE TYPE S. GROUT SHALL FOLLOW CONCRETE REQUIREMENTS, EXCEPT FOR SLUMP REQUIREMENT.
2. REQUIRED DESIGN STRENGTH f'm = 1500 PSI; PROVIDE CONCRETE MASONRY UNITS AND GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI (N&A - SEE UBC TABLE 24c); AND TYPE S MORTAR PER UBC REQUIREMENTS. AT CONTRACTOR'S OPTION, STRENGTH MAY BE ESTABLISHED BY PRSM TEST PER UBC SECTION 2105.3.
3. USE OPEN END UNITS, BOND BEAM UNITS AT HORIZONTAL REINFORCING UNITS SHALL BE PLACED IN RUNNING BOND PATTERN, UNLESS OTHERWISE NOTED.
4. REINFORCING BARS IN MASONRY SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615, GRADE 60. LAP SPICES IN HORIZONTAL REINFORCING SHALL BE 40 DIAMETERS OR 2'-0" MINIMUM TYPICAL UNLESS OTHERWISE NOTED ON PLAN. VERTICAL REINFORCING TO BE ONE PIECE, NO SPICE, UNLESS OTHERWISE NOTED ON PLANS.
5. LOW LIFT CONSTRUCTION, MAXIMUM GROUT POUR HEIGHT IS 4 FEET.
6. ALL CELLS IN CONCRETE BLOCKS SHALL BE FILLED SOLID WITH GROUT, EXCEPT AS NOTED IN THE DRAWINGS OR SPECIFICATIONS.
7. CELLS SHALL BE IN VERTICAL ALIGNMENT. DOWELS SHALL BE SET TO ALIGN WITH CORES CONTAINING REINFORCING STEEL.
8. ALL ISOLATED BOLTS EMBEDDED IN MASONRY SHALL BE GROUTED SOLIDLY IN PLACE WITH NOT LESS THAN 2" OF GROUT SURROUNDING THE BOLT.
9. REFER TO ARCHITECTURAL DRAWINGS FOR SURFACE AND HEIGHT OF UNITS, LAYING PATTERN AND JOINT TYPE.
10. CONTINUOUS SPECIAL INSPECTION AND TESTING IS REQUIRED FOR ALL MASONRY WORK (WHERE APPLICABLE).

CARPENTRY:

- 1. ALL LUMBER SHALL BE NO. 2 DOUGLAS FIR-LARCH (WCLIP OR WPA) EXCEPT:
A. INTERIOR STUDS: STANDARD OR BETTER.
B. BEAMS, POSTS AND STRINGERS: NO. 1 DOUGLAS FIR-LARCH, UNLESS NOTED OTHERWISE ON PLANS.
MAXIMUM MOISTURE CONTENT SHALL BE 19 PERCENT.
2. GLUE-LAMINATED TIMBER BEAMS:
A. CONFORM TO U.S. VOLUNTARY PRODUCT STANDARD PS 56-73.
B. LUMBER: EXTERIOR AND EXPOSED TO VIEW: ALASKAN YELLOW CEDAR
Fb = 2400 PSI
Fv = 165 PSI
E = 1800 KSI
OTHER: DOUGLAS FIR-LARCH ATC COMBINATION 24F-V4,
Fb = 2400 PSI
Fv = 165 PSI
E = 1800 KSI
AT CONT. AND CANTILEVER CONDITIONS, PROVIDE MEMBERS W/ ALLOW. STRESS PROPERTIES EQUAL TO ATC COMBINATION 24F-V8.
C. ADHESIVE: WET CONDITION OF SERVICE
D. APPEARANCE: INDUSTRIAL GRADE, EXCEPT ARCH. APPEARANCE GRADE WHERE EXPOSED TO VIEW.
E. PROTECTION: SEAL ENDS AND BUNDLE WRAP
F. PROVIDE ATC QUALITY MARK ON EACH MEMBER AND CERTIFICATE OF CONFORMANCE WITH ANSI/ATC A190.1.1983.
G. GLU-LAMINATED TIMBER BEAMS TO BE PROVIDED W/ NO CAMBERS, TYP., U.O.N.
3. ALL LUMBER IN CONTACT WITH CONCRETE, MASONRY OR EXPOSED TO WEATHER SHALL BE PRESERVATIVE PRESURE TREATED, U.O.N.
4. ALL JOIST HANGERS AND FRAMING HARDWARE SHALL BE SIMPSON "STRONG-TIE" OR APPROVED EQUIVALENT. WHERE CONNECTION IS NOT DETAILED, PROVIDE APPROPRIATE CONNECTOR PER MANUFACTURER'S RECOMMENDATION.
5. BOLTS FASTENING WOOD MEMBERS SHALL BE FITTED WITH STANDARD CUT WASHER AGAINST NUT AND BOLT HEAD. HOLES FOR BOLTS SHALL BE BORED 1/32" MAXIMUM OVERSIZE. RETIENON ALL BOLTS BEFORE CLOSING IN.
6. FOUNDATION PLATES FOR ALL EXTERIOR WALLS, SHEAR WALLS AND BEARING WALLS SHALL BE BOLTED WITH 5/8-INCH DIAMETER ANCHOR BOLTS AT 4'-0" O.C. MAXIMUM, UNLESS NOTED OTHERWISE ON FOUNDATION PLANS AND SHEARWALL SCHEDULE. BOLTS SHALL BE EMBEDDED AT LEAST SEVEN INCHES INTO CONCRETE, PROVIDE A MINIMUM OF TWO BOLTS PER PIECE LOCATED NOT MORE THAN 12 INCHES FROM EACH END.
7. DOUBLE TOP PLATES ON ALL EXTERIOR AND BEARING PARTITIONS SHALL LAP 4'-0" MINIMUM AT SPICES AND HAVE 6-164 MINIMUM AT SPLICE, U.O.N.
8. BLOCKING AND BRIDGING:
A. PROVIDE SOLID BLOCKING BETWEEN JOISTS AND RAFTERS OVER SUPPORTS.
B. PROVIDE BRIDGING BETWEEN ROOF JOISTS AT 10'-0" O.C. MAXIMUM.
C. PROVIDE SOLID BLOCKING FOR FULL DEPTH OF FLOOR JOISTS AT 10'-0" O.C. MAXIMUM.
D. PROVIDE 2X SOLID BLOCKING BETWEEN STUDS AT MID-HEIGHT IN WALLS 8'-0" HIGH AND HIGHER.
9. PARTITIONS PARALLEL TO JOISTS: PROVIDE DOUBLE JOIST UNDER PARTITIONS.
10. NAILING SCHEDULE: FOLLOW NAILING SCHEDULE AS SET FORTH IN TABLE 25-0 OF THE LATEST UNIFORM BUILDING CODE. NAILS MAY BE BOX OR COMMON WIRE, AS ALLOWED IN FOOTNOTES OF TABLE 25-0. NAILS CALLED FOR ON PLANS AND DETAILS SHALL BE COMMON WIRE. HOT-DIPPED GALVANIZED NAILS SHALL BE USED WHERE EXPOSED TO WEATHER.
11. LEAD HOLES FOR LAG SCREWS SHALL BE 1/16" LESS THAN THE DIAMETER OF THE THREADED PORTION OF SCREW.
12. PLYWOOD SHEATHING:
A. GENERAL: EACH PANEL SHALL BE IDENTIFIED WITH THE GRADE-TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION AND SHALL MEET THE REQUIREMENTS OF THE U.S. PRODUCT STANDARD PS 1. DO NOT USE ANY PLYWOOD PIECE WITH LEAST DIMENSION OF LESS THAN 2'-0".
ROOF - 15/32" PLYWOOD SHALL BE: APA RATED SHEATHING, C-D EXTERIOR. PLYWOOD NAILING SHALL BE WITH 10d NAILS SPACED AS FOLLOWS: EDGE NAIL @ 6" o.c. AND FIELD NAIL @ 12" o.c., NO BLOCKING IS REQUIRED. SEE PLAN FOR PLYWOOD LAYOUT.
FLOORS - 23/32" PLYWOOD SHALL BE APA RATED STURD-I-FLOOR T&E EXTERIOR. PLYWOOD NAILING SHALL BE WITH 10d NAILS SPACED AS FOLLOWS: EDGE NAIL @ 6" o.c. AND FIELD NAIL @ 12" o.c., NO BLOCKING IS REQUIRED. SEE PLAN FOR PLYWOOD LAYOUT. APPLY GLUE BETWEEN FLOOR FRAMING MEMBERS AND PLYWOOD PRIOR TO NAILING.
B. FLOOR AND ROOF: INSTALL PLYWOOD CONTINUOUS OVER TWO OR MORE SPANS, WITH FACE GRAIN ACROSS SUPPORTS. STAGGERED END PANEL JOINTS. ALLOW 1/16-INCH SPACING AT PANEL ENDS AND 1/8-INCH SPACING AT PANEL EDGES.
C. PANEL SHEATHED SHEAR WALLS: APA RATED SHEATHING, (PLYWOOD OR O.S.B.) EXTERIOR. SEE PLANS AND SHEAR WALL SCHEDULE FOR PANEL GRADE, THICKNESS AND NAILING REQUIREMENTS. INSTALL HORIZONTALLY OR VERTICALLY OVER TWO OR MORE SUPPORTS. ALLOW SAME SPACING BETWEEN PANELS AS INDICATED ABOVE FOR FLOOR AND ROOF. BLOCK ALL EDGES.

REINFORCING STEEL:

- 1. ALL REINFORCING STEEL SHALL BE PLACED IN CONFORMANCE WITH "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318 LATEST APPROVED EDITION), AND THE "ACI DETAILING MANUAL" AS MODIFIED BY THE PROJECT DRAWINGS AND SPECIFICATIONS.
2. REINFORCING STEEL TO BE ASTM A615, GRADE 60 DEFORMED BARS, GRADE 40 FOR #3 UNLESS NOTED. WELDED WIRE FABRIC TO BE ASTM A185.
3. ALL LAP SPICES SHALL BE 40 DIAMETERS OR 2'-0" MINIMUM UNLESS OTHERWISE NOTED ON SCHEDULE. MAINTAIN 1-1/2" CLEAR MINIMUM BETWEEN PARALLEL BARS.
4. ALL REINFORCING STEEL AND EMBEDMENTS TO BE HELD SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO ALLOW WALKING ON REINFORCEMENT. NO BRICK OR POROUS MATERIAL SHALL BE USED TO SUPPORT REINFORCING.
5. WELDING OF REINFORCING IS PROHIBITED UNLESS APPROVED BY STRUCTURAL ENGINEER.
6. REINFORCEMENT SHALL BE PLACED IN RELATIVE POSITION SHOWN ON THE DRAWINGS. NO SPICES IN REINFORCING WILL BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
7. PROVIDE FOUNDATION DOWELS TO MATCH SIZE AND SPACING OF WALL OR COLUMN REINFORCEMENT. EXTEND DOWELS 50 DIAMETER MINIMUM INTO WALL OR COLUMN AND TERMINATE WITH STANDARD HOOK 3" ABOVE BOTTOM OF FOOTING, UNLESS NOTED OTHERWISE. PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE.
8. MINIMUM COVER TO REINFORCING: 3" WHERE CONCRETE IS POURED AGAINST EARTH; 2" WHERE EXPOSED TO EARTH BUT PLACED IN FORMS; 2" WHEN EXPOSED TO WEATHER; 3/4" FOR STRUCTURAL SLAB, 3/4" FOR WALLS AND SLABS ABOVE GRADE NOT EXPOSED TO WEATHER; 1-1/2" FOR BEAMS AND COLUMNS ABOVE GRADE.
9. ALL SLABS ON GRADE SHALL HAVE JOISTS INSTALLED TO PROVIDE APPROXIMATELY 20-SQ-FOOT SQUARES, UNLESS DETAILED OTHERWISE ON THE PLANS. WHERE CONCRETE POURS ARE STOPPED, THE JOINT SHALL BE FORMED PER TYPICAL CONSTRUCTION JOINT DETAIL.
10. UNLESS DETAILED OTHERWISE, REINFORCING STEEL IN CONTINUOUS GRADE BEAMS SHALL HAVE THE TOP STEEL SPICED AT MID-SPAN AND THE BOTTOM STEEL SPICED OVER SUPPORTS (30 DIA. MIN.). AT DISCONTINUOUS ENDS, THE TOP STEEL SHALL BE BENT DOWN 12 DIA. OR 12" MIN., WHICHEVER IS GREATER. SEE SCHEDULE.
11. REINFORCE ALL ARCHITECTURAL CONCRETE WITH WELDED WIRE FABRIC: 6x6 W1.4XW1.4 MINIMUM, U.O.N.

TESTING AND INSPECTION:

- 1. PROVIDE "SPECIAL INSPECTIONS" FOR ALL ITEMS AS REQUIRED BY THE 1995 CALIFORNIA BUILDING CODE WITH 1994 UNIFORM BUILDING CODE, SECTIONS 308 AND 1707 INCLUDING THE FOLLOWING:
A) CONCRETE: DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.
B) ANCHOR BOLTS AND REINFORCING DOWELS EPOXIED INTO EXISTING CONCRETE: DURING THE INSTALLATION OF DOWELS AND THE PLACING OF CONCRETE OR EPOXY AROUND SUCH DOWELS. (PROVIDE TENSION TABLE FOR TESTING REQUIREMENTS)
C) REINFORCING STEEL: DURING PLACING OF REINFORCING STEEL.
D) GROUTING OF COLUMN BASE PLATES.
E) GROUTED ANCHORS.
F) DRILLED PIERS.
G) HOLD-DOWN INSTALLATION.
H) SHOP AND FIELD WELDING.
2. SPECIAL INSPECTION SHALL BE PERFORMED BY AN APPROVED INDEPENDENT TESTING AND INSPECTION AGENCY PROVIDED BY THE UCSF/FACILITIES MANAGEMENT INSPECTION DEPARTMENT'S INSPECTOR. COPIES OF THE TESTING AND INSPECTION REPORT SHALL BE SENT TO THE UNIVERSITY'S BUILDING/INSPECTION DEPARTMENT, ARCHITECT, STRUCTURAL ENGINEER AND OWNER.

GENERAL:

- 1. THESE GENERAL NOTES APPLY UNLESS SPECIFICALLY NOTED OTHERWISE.
2. ALL CONSTRUCTION, TESTING AND INSPECTION SHALL CONFORM TO THE CALIFORNIA BUILDING CODE, 1995 EDITION.
3. ALL DETAILS ARE TYPICAL. FOR CONDITIONS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN. SUBJECT TO REVIEW.
4. VERIFY ALL EXISTING FEATURES AND CONDITIONS (DIMENSIONS, ELEVATIONS, ETC.) PRIOR TO START OF CONSTRUCTION.
5. OMISSIONS OR DISCREPANCIES BETWEEN THE VARIOUS ELEMENTS OF THE CONTRACT DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK.
6. CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL PERMITS FOR WORK SHOWN ON THESE DRAWINGS AND ANY SHORING, UNDERPINNING, BRACING OR SCAFFOLDING REQUIRED TO COMPLETE THIS WORK.
7. SEE ARCHITECTURAL DRAWINGS FOR ITEMS AND/OR DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS. COORDINATE ARCHITECTURAL WORK WITH THIS STRUCTURAL WORK. OPENINGS IN FLOORS, BEAMS, OR JOISTS LARGER THAN THOSE SHOWN ON TYPICAL DETAILS OF STRUCTURAL DRAWINGS SHALL BE REVIEWED BY STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK.
8. DURING THE CONSTRUCTION PERIOD, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF THE BUILDING. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING, BRACING, AND GUTS IN ACCORDANCE WITH ALL NATIONAL, STATE AND LOCAL SAFETY ORDINANCES.
9. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS PER SPECIFICATIONS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
10. DESIGN LIVE LOADS:
FLOORS: 40 PSF + 20 PSF PARTITION; 100 PSF AT CORRIDOR ROOFS (REDUCIBLE)
COURTYARD: 100 PSF
11. LATERAL LOADS:
WIND: BASIC WIND SPEED 70 MPH EXPOSURE B
SEISMIC: V = 0.138W (ZONE 4)

FOUNDATION:

- 1. FOUNDATION DESIGN IS BASED ON SOILS REPORTS BY TREADWELL & ROLLO, INC. 555 MONTGOMERY ST., SUITE 1300, SAN FRANCISCO, CA 94111. TEL: (415) 955-9040. PROJECT NUMBER: 2029.01 DATED NOVEMBER 8, 1996.
2. SOILS ENGINEER SHALL VERIFY CONDITION AND/OR ADEQUACY OF ALL SURGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOOTINGS, SLABS, FILLS AND BACKFILLS, ETC. PROVIDE THE SOILS ENGINEER REPORT TO UCSF/FACILITIES MANAGEMENT INSPECTION DEPARTMENT.
3. SIDES OF FOUNDATIONS SHOWN STRAIGHT ARE FORMED. FOUNDATIONS POURED AGAINST THE EARTH AT CONTRACTOR'S OPTION REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY SOILS ENGINEER AND BE RESPONSIBLE FOR CLEAN-UP OF SLOUING BEFORE, DURING AND AFTER PLACING CONCRETE. INCREASE FOOTING DIMENSION BY 2 INCHES MINIMUM.
4. CONTRACTOR TO PROVIDE FOR DE-WEATERING OF EXCAVATION FOR EITHER SURFACE WATER, GROUND WATER OR SEEPAGE, IF REQUIRED.
5. BACKFILL OVER-EXCAVATED FOOTINGS WITH CONCRETE OF SAME DESIGN STRENGTH AS FOOTING CONCRETE.
6. NO CONCRETE SHALL BE POURED IN ANY FOUNDATION UNTIL EXCAVATION HAS BEEN INSPECTED AND APPROVED BY THE ENGINEER.
7. FOUNDATION TYPE:
SPREAD FOOTINGS:
ALLOWABLE BEARING PRESSURES:
3000 PSF DL
4500 PSF DL + LL
6000 PSF DL + LL + WIND/SEISMIC
DRILLED PIERS:
SKIN FRICTION
1000 PSF, SEE SOILS REPORT
8. STEP CONTINUOUS FOOTINGS AT VARYING ELEVATIONS PER TYPICAL DETAIL. SLOPING FOOTINGS IS PROHIBITED.
9. ALL FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOIL. BOTTOM OF FOOTINGS SHOWN ON THE DRAWINGS ARE MINIMUM AND SHALL BE LOWERED AS REQUIRED TO REMOVE SOFT AND LOOSE MATERIAL.

CONCRETE:

- 1. ALL CONCRETE SHALL BE MIXED AND PLACED IN ACCORDANCE WITH ACI 318. USE MAXIMUM AGGREGATE SIZE APPROPRIATE FOR FORM AND REBAR CLEARANCES TO BE ENCOUNTERED. (SEE A.C.I.)
2. THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE OWNER'S TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S. SUBMIT TEST DATA ON EACH PROPOSED MIX FOR REVIEW IN ACCORDANCE WITH 1994 UBC SECTION 2605. MIX DESIGNS SUBMITTED WITHOUT THE REQUIRED TEST DATA WILL BE RETURNED WITHOUT REVIEW.
3. NOMINAL WEIGHT (145 PCF) STONE AGGREGATE CONCRETE SHALL HAVE THE FOLLOWING 28-DAYS STRENGTHS (MINIMUM fc'):
FOOTINGS: 3000 PSI
DRILLED PIERS: 3000 PSI
GRADE BEAMS: 4000 PSI
WALLS: 4000 PSI
SLABS ON GRADE: 3000 PSI
EXTERIOR FLOORWORK: 3500 PSI
ALL OTHER CONCRETE, U.O.N.: 3000 PSI
4. SCHEDULING OF WORK MAY REQUIRE DESIGN STRENGTH IN SHORTER PERIODS OF TIME (LESS THAN 28 DAYS).
5. PORTLAND CEMENT SHALL CONFORM TO A.S.T.M. C-150 TYPE I OR II, LOW ALKALI, SEE SOILS REPORT.
6. AGGREGATE FOR HARDROCK CONCRETE SHALL CONFORM TO ALL REQUIREMENT SAND TESTS OF A.S.T.M. C-33 AND PROJECT SPECIFICATIONS. EXCEPTIONS MAY BE USED ONLY WITH PERMISSION OF THE STRUCTURAL ENGINEER. USE Limestone COARSE AGGREGATE FOR STRUCTURAL SLAB AND SLAB ON GRADE CONSTRUCTION.
7. AGGREGATE FOR LIGHTWEIGHT CONCRETE SHALL CONFORM TO A.S.T.M. C-330.
8. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY SAND BLASTING OR MECHANICAL MEANS. CLEAN BEFORE NEW POUR. LOCATION TO BE APPROVED BY THE STRUCTURAL ENGINEER.
9. ALL CONCRETE TO BE REINFORCED UNLESS SPECIFICALLY NOTED "NOT REINFORCED".
10. MAXIMUM SLUMP SHALL BE FOUR INCHES, UNLESS NOTED OTHERWISE.
11. GROUT UNDER BASEPLATES, SILL PLATES, ETC. SEE SPECIFICATIONS.
12. CONDUIT OR PIPE SIZE (O.D.) SHALL NOT EXCEED 30% OF SLAB THICKNESS, AND SHALL BE PLACED FOUR DIAMETERS MINIMUM APART, UNLESS SPECIFICALLY DETAILED OTHERWISE.
13. PROJECTING CORNERS OF BEAMS, WALLS, COLUMNS, ETC., SHALL BE FORMED WITH A 3/4" CHAMFER, UNLESS OTHERWISE NOTED ON ARCHITECTURAL DRAWINGS.
14. PRIOR TO PLACING CONCRETE, THE CONTRACTOR SHALL ENSURE THAT ALL EMBEDMENTS, INCLUDING COLUMN ANCHOR BOLTS, ARE PROPERLY LOCATED AND SECURELY TIED IN PLACE.
15. SUBMIT LOCATION PLAN FOR ALL PROPOSED JOINTS NOT INDICATED ON DRAWINGS FOR APPROVAL PRIOR TO WORK.
16. ALL CONCRETE REINFORCEMENT AND PLACEMENT SHALL BE INSPECTED BY THE UCSF/FACILITIES MANAGEMENT INSPECTION DEPARTMENT: INSPECTOR OF RECORD AND TESTING LABORATORY INSPECTOR.



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Gordon H Chong & Partners Architecture

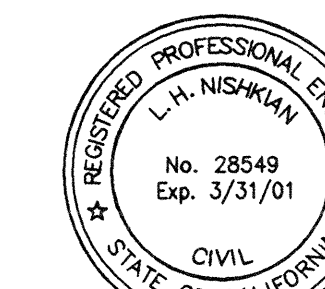
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DO NOT SCALE DRAWING. ALL MARKED DIMENSIONS TAKE PRECEDENT. ALL DIMENSIONS TO BE VERIFIED IN THE FIELD BY CONTRACTOR PRIOR TO EXECUTION OF WORKS.

Table with 3 columns: NO., DESCRIPTION, DATE. Contains revision history for the drawing.

APPROVAL STAMPS

PROJECT TITLE: ALDEA HOUSING

DRAWING TITLE: GENERAL NOTES

Table with 2 columns: PROJECT NO., FILE NO., SHEET NO. Contains project identification numbers and sheet number S01.



DATE: 7/25/97