



**FORM 1**  
**CERTIFICATE OF SEISMIC PERFORMANCE LEVEL**

- UC-Designed & Constructed Facility**  
 **Campus-Acquired or Leased Facility**

**BUILDING DATA**

Building Name: Aldea San Miguel 14  
Address: 80 Behr Ave, San Francisco  
Site location coordinates: Latitude 37.7585 Longitudinal -122.4546

**UCOP SEISMIC PERFORMANCE LEVEL (OR "RATING"): III**

ASCE 41-17 Model Building Type:

- a. Longitudinal Direction: W1: Wood Frame with Wood Shear Panels
- b. Transverse Direction: W1: Wood Frame with Wood Shear Panels

Gross Square Footage: 11,592  
Number of stories *above* grade: 3  
Number of basement stories *below* grade: 0

Year Original Building was Constructed: 1999  
Original Building Design Code & Year: CBC - 1995  
Retrofit Building Design Code & Code (if applicable): NA

**SITE INFORMATION**

Site Class: C Basis: (NA, June 2019, Pg. 1-3)  
Geologic Hazards:  
Fault Rupture: No Basis: UCSF Presumptive Buildings – Geotechnical Assessment, Egan (2019)  
Liquefaction: No Basis: UCSF Presumptive Buildings – Geotechnical Assessment, Egan (2019)  
Landslide: No Basis: UCSF Presumptive Buildings – Geotechnical Assessment, Egan (2019)

**ATTACHMENT**

Original Structural Drawings: (Aldea Housing, Nishkian & Assoc. Inc, 7/25/97, S0.1) or  
Seismic Evaluation: NA  
Retrofit Structural Drawings: NA



## CERTIFICATION & PRESUMPTIVE RATING VERIFICATION STATEMENT

I, [Maryann T. Phipps](#), a California-licensed structural engineer, am responsible for the completion of this certificate, and I have no ownership interest in the property identified above. My scope of review to support the completion of this certificate included both of the following ("No" responses must include an explanation):

- a) the review of structural drawings indicating that they are as-built or record drawings, or that they otherwise are the basis for the construction of the building:  Yes  No
- b) visiting the building to verify the observable existing conditions are reasonably consistent with those shown on the structural drawings:  Yes  No

Based on my review, I have verified that the UCOP Seismic Performance Level (SPL) is presumptively permitted by the following UC Seismic Program Guidebook provision (choose one of the following):

- 1) Contract documents indicate that the original design and construction of the aforementioned building is in accordance with the benchmark design code year (or later) building code seismic design provisions for UBC or IBC listed in Table 1 below.
- 2) The existing SPL rating is based on an acceptable basis of seismic evaluation completed in 2006 or later.
- 3) Contract documents indicate that a comprehensive<sup>1</sup> building seismic retrofit design was fully-constructed with an engineered design based on the 1997 UBC/1998 **or later** CBC, and (choose one of the following):
  - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1E (or BSE-R) and BSE-2E (or BSE-C) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 CBC **or later** for EXISTING buildings, and is presumptively assigned an SPL rating of IV.
  - the retrofit project was completed by the UC campus. Further, the design was based on ground motion parameters, at a minimum, corresponding to BSE-1 (or BSE-1N) and BSE-2 (or BSE-2N) as defined in ASCE 41, or the full design basis ground motion required in the 1997 UBC/1998 **or later** CBC for NEW buildings, and is presumptively assigned an SPL rating of III.
  - the retrofit project was not completed by the UC campus following UC policies, and is presumptively assigned an SPL rating of IV.

<sup>1</sup> A comprehensive retrofit addresses the entire building structural system as indicated by the associated seismic evaluation, as opposed to addressing selective portions of the structural system.

Campus: UCSF  
Building Name: Aldea San Miguel 14  
CAAN ID: 3019  
Auxiliary Building ID: NA



UNIVERSITY  
OF  
CALIFORNIA

Date: 8/21/2019

**CERTIFICATION SIGNATURE**

AFFIX SEAL HERE

Maryann T. Phipps  
Print Name

President  
Title

S2995  
CA Professional Registration No.

6/30/2020  
License Expiration Date

*Maryann T. Phipps*  
Signature

8/21/2019  
Date



Estructure, (510) 235-3116, 1144 65th St Suite A, Oakland  
Firm Name, Phone Number, and Address



**Table 1: Benchmark Building Codes and Standards**

Building Type <sup>a,b</sup>	Building Seismic Design Provisions	
	UBC	IBC
Wood frame, wood shear panels (Types W1 and W2)	1976	2000
Wood frame, wood shear panels (Type W1a)	1976	2000
Steel moment-resisting frame (Types S1 and S1a)	1997	2000
Steel concentrically braced frame (Types S2 and S2a)	1997	2000
Steel eccentrically braced frame (Types S2 and S2a)	1988 <sup>g</sup>	2000
Buckling-restrained braced frame (Types S2 and S2a)	f	2006
Metal building frames (Type S3)	f	2000
Steel frame with concrete shear walls (Type S4)	1994	2000
Steel frame with URM infill (Types S5 and S5a)	f	2000
Steel plate shear wall (Type S6)	f	2006
Cold-formed steel light-frame construction—shear wall system (Type CFS1)	1997 <sup>h</sup>	2000
Cold-formed steel light-frame construction—strap-braced wall system (Type CFS2)	f	2003
Reinforced concrete moment-resisting frame (Type C1) <sup>i</sup>	1994	2000
Reinforced concrete shear walls (Types C2 and C2a)	1994	2000
Concrete frame with URM infill (Types C3 and C3a)	f	f
Tilt-up concrete (Types PC1 and PC1a)	1997	2000
Precast concrete frame (Types PC2 and PC2a)	f	2000
Reinforced masonry (Type RM1)	1997	2000
Reinforced masonry (Type RM2)	1994	2000
Unreinforced masonry (Type URM)	f	f
Unreinforced masonry (Type URMa)	f	f
Seismic isolation or passive dissipation	1991	2000

Note: This table has been adapted from ASCE 41-17 Table 3-2. Benchmark Building Codes and Standards for Life Safety Structural Performed at BSE-1E.

Note: UBC = Uniform Building Code. IBC = International Building Code.

<sup>a</sup> Building type refers to one of the common building types defined in Table 3-1 of ASCE 41-17.

<sup>b</sup> Buildings on hillside sites shall not be considered Benchmark Buildings.

<sup>c</sup> not used

<sup>d</sup> not used

<sup>e</sup> not used

<sup>f</sup> No benchmark year; buildings shall be evaluated in accordance with Section III.J.

<sup>g</sup> Steel eccentrically braced frames with links adjacent to columns shall comply with the 1994 UBC Emergency Provisions, published September/October 1994, or subsequent requirements.

<sup>h</sup> Cold-formed steel shear walls with wood structural panels only.

<sup>i</sup> Flat slab concrete moment frames shall not be considered Benchmark Buildings.

ABBREVIATION:

Table of abbreviations and their corresponding full names, including ARCHITECTURAL, CONSTRUCTION, DETAIL, FINISH, GAUGE, HORIZONTAL, INSIDE, JOIST, LONG, MAXIMUM, MECH., MINIMUM, MISCELLANEOUS, NOT IN CONTRACT, OUTSIDE, OPPOSITE, POWER, PLYWOOD, REINFORCED, REDWOOD, SEE, SELF-DRILLING, SIMILAR, SPECIFICATIONS, STANDARD, STAGGERED, SYMMETRICAL, TOP OF PLATE, VERTICAL, WORK POINT, etc.

MANUFACTURED TRUSSES:

- 1. MATERIALS, DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES (TP), LATEST EDITION, THE 1995 CALIFORNIA BUILDING CODE, AND ANY APPLICABLE LOCAL CODES.
2. DESIGN LOADS: AS SHOWN ON SHEET S5.2
3. A COMPLETE SHOP DRAWING SUBMITTAL SHALL BE MADE TO THE ARCHITECT PRIOR TO FABRICATION...

STRUCTURAL STEEL:

- 1. STEEL USAGE:
A. ALL ROLLED SHAPES AND PLATES, ASTM A36 OR ASTM A572, GR.50
B. TUBE: ASTM A500, GRADE B
C. BOLTS: ASTM A307 (M.B.)
D. SHEAR CONNECTORS: ASTM A108
E. ANCHOR BOLTS: ASTM A307 (A108)
F. THREADED RODS: ASTM A36
2. ALL STRUCTURAL STEEL TO BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.

CONCRETE UNIT MASONRY (CMU):

- 1. CONCRETE BLOCK SHALL BE HOLLOW, LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C90 GRADE N111. MORTAR SHALL BE TYPE S. GROUT SHALL FOLLOW CONCRETE REQUIREMENTS, EXCEPT FOR SLUMP REQUIREMENT.
2. REQUIRED DESIGN STRENGTH f'm = 1500 PSI; PROVIDE CONCRETE MASONRY UNITS AND GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI (N8a - SEE UBC TABLE 24c) AND TYPE S MORTAR PER UBC REQUIREMENTS. AT CONTRACTOR'S OPTION, STRENGTH MAY BE ESTABLISHED BY PRSM TEST PER UBC SECTION 2105.3.

CARPENTRY:

- 1. ALL LUMBER SHALL BE NO. 2 DOUGLAS FIR-LARCH (WCLIP OR WPA) EXCEPT:
A. INTERIOR STUDS: STANDARD OR BETTER.
B. BEAMS, POSTS AND STRINGERS: NO. 1 DOUGLAS FIR-LARCH, UNLESS NOTED OTHERWISE ON PLANS.
MAXIMUM MOISTURE CONTENT SHALL BE 19 PERCENT.
2. GLUE-LAMINATED TIMBER BEAMS:
A. CONFORM TO U.S. VOLUNTARY PRODUCT STANDARD PS 56-73.
B. LUMBER: EXTERIOR AND EXPOSED TO VIEW: ALASKAN YELLOW CEDAR
Fb = 2400 PSI
Fv = 165 PSI
E = 1800 KSI
OTHER: DOUGLAS FIR-LARCH ATC COMBINATION 24F-V4,
Fb = 2400 PSI
Fv = 165 PSI
E = 1800 KSI
AT CONT. AND CANTILEVER CONDITIONS, PROVIDE MEMBERS W/ ALLOW. STRESS PROPERTIES EQUAL TO ATC COMBINATION 24F-V8.
C. ADHESIVE: WET CONDITION OF SERVICE
D. APPEARANCE: INDUSTRIAL GRADE, EXCEPT ARCH. APPEARANCE GRADE WHERE EXPOSED TO VIEW.
E. PROTECTION: SEAL ENDS AND BUNDLE WRAP
F. PROVIDE ATC QUALITY MARK ON EACH MEMBER AND CERTIFICATE OF CONFORMANCE WITH ANSI/ATC A190.1.1983.
G. GLU-LAMINATED TIMBER BEAMS TO BE PROVIDED W/ NO CAMBERS, TYP., U.O.N.

REINFORCING STEEL:

- 1. ALL REINFORCING STEEL SHALL BE PLACED IN CONFORMANCE WITH "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318 LATEST APPROVED EDITION), AND THE "ACI DETAILING MANUAL" AS MODIFIED BY THE PROJECT DRAWINGS AND SPECIFICATIONS.
2. REINFORCING STEEL TO BE ASTM A615, GRADE 60 DEFORMED BARS, GRADE 40 FOR #3 UNLESS NOTED. WELDED WIRE FABRIC TO BE ASTM A185.
3. ALL LAP SPICES SHALL BE 40 DIAMETERS OR 2'-0" MINIMUM UNLESS OTHERWISE NOTED ON SCHEDULE. MAINTAIN 1-1/2" CLEAR MINIMUM BETWEEN PARALLEL BARS.
4. ALL REINFORCING STEEL AND EMBEDMENTS TO BE HELD SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO ALLOW WALKING ON REINFORCEMENT. NO BRICK OR POROUS MATERIAL SHALL BE USED TO SUPPORT REINFORCING.
5. WELDING OF REINFORCING IS PROHIBITED UNLESS APPROVED BY STRUCTURAL ENGINEER.

TESTING AND INSPECTION:

- 1. PROVIDE "SPECIAL INSPECTIONS" FOR ALL ITEMS AS REQUIRED BY THE 1995 CALIFORNIA BUILDING CODE WITH 1994 UNIFORM BUILDING CODE, SECTIONS 308 AND 1707 INCLUDING THE FOLLOWING:
A) CONCRETE: DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.
B) ANCHOR BOLTS AND REINFORCING DOWELS EPOXIED INTO EXISTING CONCRETE: DURING THE INSTALLATION OF DOWELS AND THE PLACING OF CONCRETE OR EPOXY AROUND SUCH DOWELS. (PROVIDE TENSION TABLE FOR TESTING REQUIREMENTS)
C) REINFORCING STEEL: DURING PLACING OF REINFORCING STEEL.
D) GROUTING OF COLUMN BASE PLATES.
E) GROUTED ANCHORS.
F) DRILLED PIERS.
G) HOLD-DOWN INSTALLATION.
H) SHOP AND FIELD WELDING.

GENERAL:

- 1. THESE GENERAL NOTES APPLY UNLESS SPECIFICALLY NOTED OTHERWISE.
2. ALL CONSTRUCTION, TESTING AND INSPECTION SHALL CONFORM TO THE CALIFORNIA BUILDING CODE, 1995 EDITION.
3. ALL DETAILS ARE TYPICAL. FOR CONDITIONS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN. SUBJECT TO REVIEW.
4. VERIFY ALL EXISTING FEATURES AND CONDITIONS (DIMENSIONS, ELEVATIONS, ETC.) PRIOR TO START OF CONSTRUCTION.
5. OMISSIONS OR DISCREPANCIES BETWEEN THE VARIOUS ELEMENTS OF THE CONTRACT DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK.
6. CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL PERMITS FOR WORK SHOWN ON THESE DRAWINGS AND ANY SHORING, UNDERPINNING, BRACING OR SCAFFOLDING REQUIRED TO COMPLETE THIS WORK.

FOUNDATION:

- 1. FOUNDATION DESIGN IS BASED ON SOILS REPORTS BY TREADWELL & ROLLO, INC. 555 MONTGOMERY ST., SUITE 1300, SAN FRANCISCO, CA 94111. TEL: (415) 955-9040. PROJECT NUMBER: 2029.01 DATED NOVEMBER 8, 1996.
2. SOILS ENGINEER SHALL VERIFY CONDITION AND/OR ADEQUACY OF ALL SURGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOOTINGS, SLABS, FILLS AND BACKFILLS, ETC. PROVIDE THE SOILS ENGINEER REPORT TO UCSF/FACILITIES MANAGEMENT INSPECTION DEPARTMENT.
3. SIDES OF FOUNDATIONS SHOWN STRAIGHT ARE FORMED. FOUNDATIONS POURED AGAINST THE EARTH AT CONTRACTOR'S OPTION REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY SOILS ENGINEER AND BE RESPONSIBLE FOR CLEAN-UP OF SLOUING BEFORE, DURING AND AFTER PLACING CONCRETE. INCREASE FOOTING DIMENSION BY 2 INCHES MINIMUM.
4. CONTRACTOR TO PROVIDE FOR DEE WATERING OF EXCAVATION FOR EITHER SURFACE WATER, GROUND WATER OR SEEPAGE, IF REQUIRED.
5. BACKFILL OVER-EXCAVATED FOOTINGS WITH CONCRETE OF SAME DESIGN STRENGTH AS FOOTING CONCRETE.
6. NO CONCRETE SHALL BE POURED IN ANY FOUNDATION UNTIL EXCAVATION HAS BEEN INSPECTED AND APPROVED BY THE ENGINEER.

CONCRETE:

- 1. ALL CONCRETE SHALL BE MIXED AND PLACED IN ACCORDANCE WITH ACI 318. USE MAXIMUM AGGREGATE SIZE APPROPRIATE FOR FORM AND REBAR CLEARANCES TO BE ENCOUNTERED. (SEE A.C.I.)
2. THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE OWNER'S TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S. SUBMIT TEST DATA ON EACH PROPOSED MIX FOR REVIEW IN ACCORDANCE WITH 1994 UBC SECTION 2605. MIX DESIGNS SUBMITTED WITHOUT THE REQUIRED TEST DATA WILL BE RETURNED WITHOUT REVIEW.
3. NOMINAL WEIGHT (145 PCF) STONE AGGREGATE CONCRETE SHALL HAVE THE FOLLOWING 28-DAYS STRENGTHS (MINIMUM fc'):
FOOTINGS: 3000 PSI
DRILLED PIERS: 3000 PSI
GRADE BEAMS: 4000 PSI
WALLS: 4000 PSI
SLABS ON GRADE: 3000 PSI
EXTERIOR FLOORWORK: 3500 PSI
ALL OTHER CONCRETE, U.O.N.: 3000 PSI
4. SCHEDULING OF WORK MAY REQUIRE DESIGN STRENGTH IN SHORTER PERIODS OF TIME (LESS THAN 28 DAYS).
5. PORTLAND CEMENT SHALL CONFORM TO A.S.T.M. C-150 TYPE I OR II, LOW ALKALI, SEE SOILS REPORT.
6. AGGREGATE FOR HARDROCK CONCRETE SHALL CONFORM TO ALL REQUIREMENT SAND TESTS OF A.S.T.M. C-33 AND PROJECT SPECIFICATIONS. EXCEPTIONS MAY BE USED ONLY WITH PERMISSION OF THE STRUCTURAL ENGINEER. USE Limestone COARSE AGGREGATE FOR STRUCTURAL SLAB AND SLAB ON GRADE CONSTRUCTION.
7. AGGREGATE FOR LIGHTWEIGHT CONCRETE SHALL CONFORM TO A.S.T.M. C-330.
8. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY SAND BLASTING OR MECHANICAL MEANS. CLEAN BEFORE NEW POUR. LOCATION TO BE APPROVED BY THE STRUCTURAL ENGINEER.
9. ALL CONCRETE TO BE REINFORCED UNLESS SPECIFICALLY NOTED "NOT REINFORCED".
10. MAXIMUM SLUMP SHALL BE FOUR INCHES, UNLESS NOTED OTHERWISE.
11. GROUT UNDER BASEPLATES, SILL PLATES, ETC. SEE SPECIFICATIONS.
12. CONDUIT OR PIPE SIZE (O.D.) SHALL NOT EXCEED 30% OF SLAB THICKNESS, AND SHALL BE PLACED FOUR DIAMETERS MINIMUM APART, UNLESS SPECIFICALLY DETAILED OTHERWISE.
13. PROJECTING CORNERS OF BEAMS, WALLS, COLUMNS, ETC., SHALL BE FORMED WITH A 3/4" CHAMFER, UNLESS OTHERWISE NOTED ON ARCHITECTURAL DRAWINGS.
14. PRIOR TO PLACING CONCRETE, THE CONTRACTOR SHALL ENSURE THAT ALL EMBEDMENTS, INCLUDING COLUMN ANCHOR BOLTS, ARE PROPERLY LOCATED AND SECURELY TIED IN PLACE.
15. SUBMIT LOCATION PLAN FOR ALL PROPOSED JOINTS NOT INDICATED ON DRAWINGS FOR APPROVAL PRIOR TO WORK.
16. ALL CONCRETE REINFORCEMENT AND PLACEMENT SHALL BE INSPECTED BY THE UCSF/FACILITIES MANAGEMENT INSPECTION DEPARTMENT: INSPECTOR OF RECORD AND TESTING LABORATORY INSPECTOR.



UNIVERSITY OF CALIFORNIA SAN FRANCISCO
FACILITIES MANAGEMENT
3130 20TH STREET
SAN FRANCISCO, CALIFORNIA 94143-0894

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Gordon H Chong & Partners Architecture

The Hallide Building
130 Sutter Street, Suite 300
San Francisco, CA 94104-4012
415.433.0120 tel
415.433.4368 fax

Nishkian & Assoc. Inc. Structural Engineer

1 Holland Court, Suite 100
San Francisco, CA 94103
415.541.9477 tel
415.543.5071 fax

Sandis Humber Jones Civil/Surveying

605 Castro Street
Mountain View, CA 94041-2011
415.969-6900 tel
415.969-6472 fax



DO NOT SCALE DRAWING. ALL MARKED DIMENSIONS TAKE PRECEDENT. ALL DIMENSIONS TO BE VERIFIED IN THE FIELD BY CONTRACTOR PRIOR TO EXECUTION OF WORKS.

Table with columns: NO., DESCRIPTION, DATE. Contains revision history for the drawing.

APPROVAL STAMPS

Table for approval stamps with columns: NO., NAME, TITLE, DATE.

PROJECT TITLE: ALDEA HOUSING

GENERAL NOTES

Table with project information: PROJECT NO. M4310, FILE NO. 6737, SHEET NO. S01, DATE: 7/25/97.



Vertical text on the left margin: UCSF Facilities Management, 3130 20th Street, San Francisco, CA 94143-0894, (415) 433-0120, (415) 433-4368.

Vertical text on the left margin: UCSF Facilities Management, 3130 20th Street, San Francisco, CA 94143-0894, (415) 433-0120, (415) 433-4368.